



*Tarrant Regional Water District
Watershed Project*

*Cedar Creek Reservoir
Wastewater Treatment Facilities Report*

*Evaluating Water Quality Management Practices for
Reservoirs in North Central Texas*

*Revised
May 5, 2008*





Tarrant Regional Water District Watershed Project

Cedar Creek Reservoir Wastewater Treatment Facilities Report

Table of Contents

List of Tables	iv
List of Figures.....	vii
ES. Executive Summary	ES-1
I. City of Athens	I-1
Introduction	I-1
Existing Permit Conditions	I-1
Historical Plant Flows and Effluent Quality	I-2
Process Overview	I-3
Future Flow Projections	I-6
Facility Needs Evaluation	I-6
Level I Facility Needs for 2050 Wastewater Flows	I-6
Level II Facility Needs for 2050 Wastewater Flows.....	I-7
Level III Facility Needs for 2050 Wastewater Flows	I-8
II. Cherokee Shores	II-1
Introduction	II-1
Existing Permit Conditions	II-1
Historical Plant Flows and Effluent Quality	II-2
Process Overview.....	II-4
Future Flow Projections	II-6
Facility Needs Evaluation	II-6
Level I Facility Needs for 2050 Wastewater Flows	II-6
Level II Facility Needs for 2050 Wastewater Flows.....	II-6
Level III Facility Needs for 2050 Wastewater Flows	II-7
III. East Cedar Creek FWSD North WWTP	III-1
Introduction	III-1
Existing Permit Conditions	III-1
Historical Plant Flows and Effluent Quality	III-2
Process Overview.....	III-4
Future Flow Projections	III-6
Facility Needs Evaluation	III-6
Level I Facility Needs for 2050 Wastewater Flows	III-6
Level II Facility Needs for 2050 Wastewater Flows.....	III-7
Level III Facility Needs for 2050 Wastewater Flows	III-8



IV.	City of Eustace	IV-1
	Introduction	IV-1
	Existing Permit Conditions	IV-1
	Historical Plant Flows and Effluent Quality	IV-2
	Process Overview	IV-4
	Future Flow Projections	IV-6
	Facility Needs Evaluation	IV-6
	Level I Facility Needs for 2050 Wastewater Flows	IV-6
	Level II Facility Needs for 2050 Wastewater Flows.....	IV-7
	Level III Facility Needs for 2050 Wastewater Flows	IV-8
V.	City of Kaufman	V-1
	Introduction	V-1
	Existing Permit Conditions	V-1
	Historical Plant Flows and Effluent Quality	V-2
	Process Overview	V-4
	Future Flow Projections	V-6
	Facility Needs Evaluation	V-6
	Level I Facility Needs for 2050 Wastewater Flows	V-6
	Level II Facility Needs for 2050 Wastewater Flows.....	V-7
	Level III Facility Needs for 2050 Wastewater Flows	V-8
VI.	City of Kemp	VI-1
	Introduction	VI-1
	Existing Permit Conditions	VI-1
	Historical Plant Flows and Effluent Quality	VI-2
	Process Overview	VI-4
	Future Flow Projections	VI-6
	Facility Needs Evaluation	VI-6
	Level I Facility Needs for 2050 Wastewater Flows	VI-6
	Level II Facility Needs for 2050 Wastewater Flows.....	VI-6
	Level III Facility Needs for 2050 Wastewater Flows	VI-7
VII.	City of Mabank	VII-1
	Introduction	VII-1
	Existing Permit Conditions	VII-1
	Historical Plant Flows and Effluent Quality	VII-2
	Process Overview	VII-4
	Future Flow Projections	VII-6
	Facility Needs Evaluation	VII-6
	Level I Facility Needs for 2050 Wastewater Flows	VII-6
	Level II Facility Needs for 2050 Wastewater Flows.....	VII-7
	Level III Facility Needs for 2050 Wastewater Flows	VII-8



VIII.	City of Terrell King's Creek WWTP	VIII-1
	Introduction	VIII-1
	Existing Permit Conditions	VIII-1
	Historical Plant Flows and Effluent Quality	VIII-2
	Process Overview	VIII-4
	Future Flow Projections	VIII-6
	Facility Needs Evaluation	VIII-6
	Level I Facility Needs for 2050 Wastewater Flows	VIII-6
	Level II Facility Needs for 2050 Wastewater Flows.....	VIII-7
	Level III Facility Needs for 2050 Wastewater Flows	VIII-8
IX.	City of Wills Point	IX-1
	Introduction	IX-1
	Existing Permit Conditions	IX-1
	Historical Plant Flows and Effluent Quality	IX-2
	Process Overview.....	IX-4
	Future Flow Projections	IX-6
	Facility Needs Evaluation	IX-6
	Level I Facility Needs for 2050 Wastewater Flows	IX-6
	Level II Facility Needs for 2050 Wastewater Flows.....	IX-6
	Level III Facility Needs for 2050 Wastewater Flows	IX-7

Appendices

City of Athens Nutrient Data	A-1
Cherokee Shores Nutrient Data.....	A-2
East Cedar Creek FWSD Nutrient Data.....	A-3
City of Eustace Nutrient Data.....	A-4
City of Kaufman Nutrient Data.....	A-5
City of Kemp Nutrient Data.....	A-6
City of Mabank Nutrient Data.....	A-7
City of Terrell King's Creek WWTP Nutrient Data	A-8
City of Wills Point Nutrient Data	A-9



Tarrant Regional Water District Watershed Project

Cedar Creek Reservoir Wastewater Treatment Facilities Report

List of Tables

Table ES-1:	Tarrant Regional Water District Cedar Creek Lake Wastewater Plant Dischargers Assessment	ES-3
Table ES-2:	Comparison of Costs for Each Plant's Expansion or Upgrade.....	ES-9
Table ES-3:	Nutrient Loads in Lbs/day and Tons/year for Each Plant at Each Permit Level	ES-9
Table I-1:	City of Athens North WWTP Current TPDES Permit Conditions	I-1
Table I-2:	City of Athens North WWTP Historical Plant Flows and Effluent Quality	I-2
Table I-3:	City of Athens WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	I-2
Table I-4:	City of Athens North WWTP Existing Treatment Facilities.....	I-4
Table I-5:	City of Athens Population and Flow Projections	I-6
Table I-6:	City of Athens North WWTP Facility Needs to Meet Level I Effluent Requirements and 2050 Flows	I-7
Table I-7:	City of Athens North WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	I-8
Table I-8:	City of Athens North WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	I-9
Table II-1:	Cherokee Shores WWTF Current TPDES Permit Conditions	II-1
Table II-2:	Cherokee Shores WWTF Historical Plant Flows and Effluent Quality	II-2
Table II-3:	Cherokee Shores WWTF Special Nutrient Testing by Texas A&M Agricultural Extension	II-2
Table II-4:	Cherokee Shores WWTF Existing Treatment Facilities	II-4
Table II-5:	Cherokee Shores WWTF Population and Flow Projections	II-6
Table II-6:	Cherokee Shores WWTF Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	II-7
Table II-7:	Cherokee Shores WWTF Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	II-8
Table III-1:	ECCFWSD North WWTP Current TPDES Permit Conditions	III-1
Table III-2:	ECCFWSD North WWTP Historical Plant Flows and Effluent Quality	III-2
Table III-3:	ECCFWSD WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	III-2
Table III-4:	ECCFWSD North WWTP Existing Treatment Facilities	III-4
Table III-5:	ECCFWSD Population and Flow Projections	III-6
Table III-6:	ECCFWSD North WWTP Facility Needs to Meet Level I Effluent Requirements and 2050 Flows	III-7
Table III-7:	ECCFWSD North WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	III-8



Tarrant Regional Water District Watershed Project

Cedar Creek Reservoir Wastewater Treatment Facilities Report

List of Tables, continued

Table III-8:	ECCFWSD North WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	III-9
Table IV-1:	City of Eustace WWTP Current TPDES Permit Conditions.....	IV-1
Table IV-2:	City of Eustace WWTP Historical Plant Flows and Effluent Quality.....	IV-2
Table IV-3:	City of Eustace WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	IV-2
Table IV-4:	City of Eustace WWTP Existing Treatment Facilities.....	IV-4
Table IV-5:	City of Eustace Population and Flow Projections	IV-6
Table IV-6:	City of Eustace WWTP Facility Needs to Meet Level I Effluent Requirements and 2050 Flows	IV-7
Table IV-7:	City of Eustace WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	IV-8
Table IV-8:	City of Eustace WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	IV-9
Table V-1:	City of Kaufman WWTP Current TPDES Permit Conditions	V-1
Table V-2:	City of Kaufman WWTP Historical Plant Flows and Effluent Quality	V-2
Table V-3:	City of Kaufman WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	V-2
Table V-4:	City of Kaufman WWTP Existing Treatment Facilities	V-4
Table V-5:	City of Kaufman WWTP Population and Flow Projections.....	V-6
Table V-6:	City of Kaufman WWTP Facility Needs to Meet Level I Effluent Requirements and 2050 Flows	V-7
Table V-7:	City of Kaufman WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	V-8
Table V-8:	City of Kaufman WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	V-9
Table VI-1:	City of Kemp WWTP Current TPDES Permit Conditions	VI-1
Table VI-2:	City of Kemp WWTP Historical Plant Flows and Effluent Quality	VI-2
Table VI-3:	City of Kemp WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	VI-2
Table VI-4:	City of Kemp WWTP Existing Treatment Facilities	VI-4
Table VI-5:	City of Kemp Population and Flow Projections.....	VI-6
Table VI-6:	City of Kemp WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	VI-7
Table VI-7:	City of Kemp WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	VI-8



Tarrant Regional Water District Watershed Project

Cedar Creek Reservoir Wastewater Treatment Facilities Report

List of Tables, continued

Table VII-1:	City of Mabank WWTP Current TPDES Permit Conditions.....	VII-1
Table VII-2:	City of Mabank WWTP Historical Plant Flows and Effluent Quality	VII-2
Table VII-3:	City of Mabank WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	VII-2
Table VII-4:	City of Mabank WWTP Existing Treatment Facilities	VII-4
Table VII-5:	City of Mabank Population and Flow Projections	VII-6
Table VII-6:	City of Mabank WWTP Facility Needs to Meet Level I Effluent Requirements and 2050 Flows	VII-7
Table VII-7:	City of Mabank WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	VII-8
Table VII-8:	City of Mabank WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	VII-9
Table VIII-1:	City of Terrell King's Creek WWTP Current Permit Conditions	VIII-1
Table VIII-2:	City of Terrell King's Creek WWTP Historical Plant Flows and Effluent Quality	VIII-2
Table VIII-3:	City of Terrell King's Creek WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	VIII-2
Table VIII-4:	City of Terrell King's Creek WWTP Existing Treatment Facilities	VIII-4
Table VIII-5:	City of Terrell Population and Flow Projections.....	VIII-6
Table VIII-6:	City of Terrell King's Creek WWTP Facility Needs to Meet Level I Effluent Requirements and 2050 Flows	VIII-7
Table VIII-7:	City of Terrell King's Creek WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	VIII-8
Table VIII-8:	City of Terrell King's Creek WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	VIII-9
Table IX-1:	City of Wills Point WWTP Current TPDES Permit Conditions	IX-1
Table IX-2:	City of Wills Point WWTP Historical Plant Flows and Effluent Quality	IX-2
Table IX-3:	City of Wills Point WWTP Special Nutrient Testing by Texas A&M Agricultural Extension.....	IX-2
Table IX-4A:	City of Wills Point WWTP Existing Treatment Facilities	IX-4
Table IX-4B:	City of Wills Point WWTP Planned Treatment Facilities.....	IX-4
Table IX-5:	City of Wills Point Population and Flow Projections	IX-6
Table IX-6:	City of Wills Point WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	IX-7
Table IX-7:	City of Wills Point WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	IX-8





Tarrant Regional Water District Watershed Project

Cedar Creek Reservoir Wastewater Treatment Facilities Report

List of Figures

Figure ES-1:	Cedar Creek Discharge Assessment Wastewater Treatment Plant Location	ES-2
Figure I-1:	Athens Total Nitrogen Monthly Average Loads	I-3
Figure I-2:	Athens Total Phosphorous Monthly Average Loads.....	I-3
Figure I-3:	City of Athens, Texas North Wastewater Treatment Plant, Plant Schematic	I-5
Figure II-1:	Cherokee Shores Total Nitrogen Monthly Average Loads	II-3
Figure II-2:	Cherokee Shores Total Phosphorous Monthly Average Loads.....	II-3
Figure II-3:	Tecon Water Company, L.P. Cherokee Shores Wastewater Treatment Plant, Plant Schematic	II-5
Figure III-1:	East Cedar Creek Total Nitrogen Loads.....	III-3
Figure III-2:	East Cedar Creek Total Phosphorous Loads	III-3
Figure III-3:	East Cedar Creek Fresh Water Supply District North Wastewater Treatment Plant, Plant Schematic	III-5
Figure IV-1:	Eustace Total Nitrogen Monthly Average Loads.....	IV-3
Figure IV-2:	Eustace Total Nitrogen Monthly Average Loads.....	IV-3
Figure IV-3:	City of Eustace, Texas Wastewater Treatment Plant, Plant Schematic.....	IV-5
Figure V-1:	Kaufman Total Nitrogen Monthly Average Loads	V-3
Figure V-2:	Kaufman Total Phosphorous Monthly Average Loads	V-3
Figure V-3:	City of Kaufman, Texas Wastewater Treatment Plant, Plant Schematic	V-5
Figure VI-1:	Kemp Total Nitrogen Monthly Average Loads.....	VI-3
Figure VI-2:	Kemp Total Phosphorous Monthly Average Loads	VI-3
Figure VI-3:	City of Kemp, Texas Wastewater Treatment Plant, Plant Schematic	VI-5
Figure VII-1:	Mabank Total Nitrogen Monthly Average Loads	VII-3
Figure VII-2:	Mabank Total Phosphorous Monthly Average Loads	VII-3
Figure VII-3:	City of Mabank, Texas Wastewater Treatment Plant, Plant Schematic	VII-5
Figure VIII-1:	Terrell Total Nitrogen Monthly Average Loads	VIII-3
Figure VIII-2:	Terrell Total Phosphorous Monthly Average Loads	VIII-3
Figure VIII-3:	City of Terrell, Texas King's Creek Wastewater Treatment Plant, Plant Schematic	VIII-5
Figure IX-1:	Wills Point Total Nitrogen Monthly Average Loads	IX-3
Figure IX-2:	Wills Point Total Phosphorous Monthly Average Loads.....	IX-3
Figure IX-3:	City of Wills Point, Texas Wastewater Treatment Plant, Plant Schematic	IX-5

List of Abbreviations

BOD	Biochemical Oxygen Demand/Biological Oxygen Demand
CBOD	Carbonaceous Biological Oxygen Demand
DMR	Discharge Monitoring Reports
ENR	Engineering News Record
IP	Inorganic Phosphorous
MG	Millions of Gallons
MGD	Millions of Gallons per Day
NH3	Ammonia
NH3-N	Ammonia Nitrogen
NOx	Nitrogen Oxides
ON	Organic Nitrogen
OP	Organic Phosphorous
OPO₄	Ortho-Phosphate
OrgN	Organic Nitrogen
OrgP	Organic Phosphorous
SWD	Side Wall Depth
TCEQ	Texas Commission on Environmental Quality
TN	Total Nitrogen
TP	Total Phosphorous
TPDES	Texas Pollution Discharge Elimination System
TSS	Total Suspended Solids
UV	Ultra Violet
WWTP	Wastewater Treatment Plant

EXECUTIVE SUMMARY

EXECUTIVE SUMMARY

The Tarrant Regional Water District (District) provides raw water to numerous water suppliers in the North Texas area. The raw water originates in several of the District's reservoirs including Cedar Creek, Richland Chambers, and Eagle Mountain. As the demands for raw water increase, the need to protect the water quality becomes a critical issue. This report focuses on evaluating the various point sources of pollutants, particularly nutrients, into the Cedar Creek Reservoir watersheds. The objectives of the following report are to identify significant sources of pollutant sources, to quantify both current and long-term impacts, to evaluate the best management practices available for maintaining and improving water quality, and finally to address the costs of implementing those practices.

This report addresses the point sources, specifically wastewater treatment facilities, discharging directly into the Cedar Creek Reservoir through watershed streams that eventually enter the reservoir. Plants evaluated include:

- City of Athens' North Wastewater Treatment Plant
- Cherokee Shores Wastewater Treatment Facility
- East Cedar Creek Fresh Water Supply District North Wastewater Treatment Plant
- City of Eustace Wastewater Treatment Plant
- City of Kaufman Wastewater Treatment Plant
- City of Kemp Wastewater Treatment Plant
- City of Mabank Wastewater Treatment Plant
- City of Terrell King's Creek Wastewater Treatment Plant
- City of Will's Point Wastewater Treatment Plant

Figure ES-I shows the locations of these plants within the Cedar Creek watershed. The evaluation of each facility included assessing the ability of each plant to properly treat projected 2050 flows under three varying discharge limit criteria. The existing permit conditions are referred to as Level I. The next level of treatment, referred to as Level II, includes reduction of phosphorus to 1.0 mg/L and total nitrogen to 10 mg/L. The most stringent level of treatment considered, referred to as Level III, includes reducing the phosphorus limit to 0.5 mg/L and total nitrogen to 5 mg/L. An assessment of facility needs is performed at all three effluent limitations for flows projected through 2050.

Sources of information for the evaluations included: site visits, interviews with the plant personnel, reviews of the existing plans and historical reports, data collected by plant personnel for the District, data acquired through Texas Commission on Environmental Quality (TCEQ), and responses to a questionnaire developed specifically for this project to aid in acquiring data and information on each plant. Each memorandum is organized as follows:

- Description of the existing TPDES permit conditions and projected future limits
- Historical effluent data
- Brief description of the existing treatment process
- Population growth and flow projections
- Facility needs and costs to meet Level I effluent limits through 2050
- Facility needs and costs to meet Level II effluent limits through 2050
- Facility needs and costs to meet Level III effluent limits through 2050

A brief summary of the assessment for each plant is provided in Table ES-1. Detailed evaluations of each plant are provided in the technical memoranda following this summary. Table ES-2 summarizes the costs associated with each of the expansions or upgrades and Table ES-3 examines the nitrogen and phosphorus loads that would be associated with the projected 2050 flows at each permit level. The average total nitrogen and total phosphorous concentrations that were determined as part of special nutrient testing were used to calculate the Level I loads at 2050 flows; this will best compare the impact of each permit level across time.



Table ES-1
Tarrant Regional Water District
Cedar Creek Lake
Wastewater Plant Dischargers Assessment

Treatment Plant & Permit Limits	Flow Projections (MGD) ¹	Existing Process Description	Facility Needs for 2050 and Estimates of Probable Construction and O&M Costs		
			Level I Existing Permit Limits	Level II ² TN = 10 mg/L TP = 1 mg/L	Level III ³ TN = 5 mg/L TP = 0.5 mg/L
ATHENS	North WWTP 2005 = 0.50 2010 = 0.54 2020 = 0.65 2030 = 0.78 2040 = 0.94 2050 = 1.14 West WWTP 2005 = 0.74 2010 = 0.82 2020 = 0.98 2030 = 1.18 2040 = 1.42 2050 = 1.70 Flow = 1.027 MGD	Bar screen, grit removal, Imhoff tanks, supernatant to tricking filters, aeration for six hours, final clarification, and chlorination. Sludge disposed on drying beds and periodically disposed of off-site.	Expand influent pumping system, replace tricking filter rock media with plastic media, duplicate aeration basin and aeration, and double drying bed size. Project cost: \$3,428,914	Add denitrifying filter to meet TN limit and alum addition for P removal. Increase in cost from Levels I and II: \$76,507	Add carbon source for denitrification, and increase alum dosage for lower P limit. Equipment is already in place. Increase in cost from Levels I and II: \$76,507
CHEROKEE SHORES	BOD ₅ = 10 mg/L (Mar. - Nov.) CBOD = 20 mg/L (Dec. - Feb.) TSS = 15 mg/L (Mar. - Nov.) TSS = 20 mg/L (Dec. - Feb.) NH ₃ = 2 mg/L (Mar. - Nov.) NH ₃ = 3 mg/L (Dec. - Feb.) Flow = 1.027 MGD	Equalization, activated sludge, final clarification, sand filtration, and chlorination. Sludge aerobically digested and disposed off-site.	Additional Annual O&M cost: \$63,000	Increase in Annual O&M cost over Level I: \$22,890	Increase in Annual O&M cost over Levels I and II: \$38,535
	2005 = 0.08 2010 = 0.09 2020 = 0.11 2030 = 0.13 2040 = 0.15 2050 = 0.18	Existing facility is capable of treating the projected flows through the year 2050.	Project cost: \$0 Additional Annual O&M cost: \$0	Increase in cost over Level I: \$592,351	Increase in Annual O&M cost over Levels I and II: \$13,020

Table ES-1
Tarrant Regional Water District
Cedar Creek Lake
Wastewater Plant Dischargers Assessment

Treatment Plant & Permit Limits	Flow Projections ¹ (MGD)	Existing Process Description	Facility Needs for 2050 and Probable Construction and O&M Costs		
			Level I Existing Permit Limits	Level II ² TN = 10 mg/L TP = 1 mg/L	Level III ³ TN = 5 mg/L TP = 0.5 mg/L
EAST CEDAR CREEK FWSD BOD ₅ = 10 mg/L TSS = 15 mg/L Flow = 0.626 MGD	2005 = 0.71 2010 = 0.82 2020 = 1.00 2030 = 1.17 2040 = 1.36 2050 = 1.60	Treatment facility currently under expansion. Bar screen, two oxidation ditches w/ RAS, final clarification, sand filtration, and chlorination. Sludge is dried and landfilled.	Once treatment units are optimized, expansion required between 2030 and 2040. Add oxidation ditch, clarifier, filter, and chlorine contact basin.	Operate oxidation ditch for denitrification. Alum addition for P removal.	Denitrifying filter to meet TN limit. Add carbon source for denitrification and additional alum for lower P limit.

Table ES-1
Tarrant Regional Water District
Cedar Creek Lake
Wastewater Plant Dischargers Assessment

Treatment Plant & Permit Limits	Flow Projections (MGD) ¹	Existing Process Description	Facility Needs for 2050 and Probable Construction and O&M Costs		
			Level I Existing Permit Limits	Level II ² TN = 10 mg/L TP = 1 mg/L	Level III ³ TN = 5 mg/L TP = 0.5 mg/L
EUSTACE BOD ₅ = 30 mg/L TSS = 90 mg/L Flow = 0.125 MGD	2005 = 0.084 2010 = 0.088 2020 = 0.097 2030 = 0.106 2040 = 0.115 2050 = 0.126	Bar screen, oxidation ditch w/o RAS as pre-aeration basins, two stabilization ponds w/ 21-day detention time. Disinfection not required.	Add two 15 hp aerators, clarifier, RAS/WAS pumps and piping, and disinfection system	Denitrifying filter to meet TN limit, and additional carbon source for denitrification. Additional alum for lower P limit.	Increase in cost from Level I: \$51,005
			Project cost: \$217,930 Additional Annual O&M cost: \$63,000	Increase in Annual O&M cost over Level I: \$4,683	Increase in cost from Levels I and II: \$549,461 Increase in Annual O&M cost over Levels I and II: \$30,030

Table ES-1
Tarrant Regional Water District
Cedar Creek Lake
Wastewater Plant Dischargers Assessment

Treatment Plant & Permit Limits	Flow Projections ¹ (MGD) CBOD = 10 mg/L TSS = 15 mg/L NH ₃ = 3 mg/L (May - Oct.) NH ₃ = 5 mg/L (Nov - Apr.) Flow = 1.2 MGD	Existing Process Description	Facility Needs for 2050 and Probable Construction and O&M Costs		
			Level I Existing Permit Limits	Level II ² TN = 10 mg/L TP = 1 mg/L	Level III ³ TN = 5 mg/L TP = 0.5 mg/L
KAUFMAN	2005 = 0.73 2010 = 0.83 2020 = 1.09 2030 = 1.30 2040 = 1.47 2050 = 1.65	Bar screen, grit removal, storm water holding basins, two aeration basins w/ RAS, two final clarifiers, sand filtration and ultraviolet disinfection. Sludge belt press and lime blending. Sludge disposed offsite	Convert existing aeration system to fine bubble diffusers instead of adding aeration basins. Expand WAS and RAS system. Add IFAS to aeration basins, add final clarifier, and additional ultraviolet disinfection equipment.	Denitrifying filter to meet TN limit. Alum addition required for P removal.	Add carbon source for denitrification, and additional alum for lower P limit.
KEMP	2005 = 0.113 2010 = 0.113 2020 = 0.113 2030 = 0.113 2040 = 0.113 2050 = 0.113	Bar screen, grit removal, oxidation ditch w/ RAS, two final clarifiers, and chlorination. Sludge drying beds.	Existing facility is capable of treating the projected flows through the year 2050.	Operate oxidation ditch for denitrification. Alum addition for P removal. May require additional drying beds for alum sludge.	Increase in cost from Levels I and II: \$92,736

Table ES-1
Tarrant Regional Water District
Cedar Creek Lake
Wastewater Plant Dischargers Assessment

Treatment Plant & Permit Limits	Flow Projections (MGD) ¹	Existing Process Description	Facility Needs for 2050 and Probable Construction and O&M Costs		
			Level I Existing Permit Limits	Level II ² TN = 10 mg/L TP = 1 mg/L	Level III ³ TN = 5 mg/L TP = 0.5 mg/L
MABANK CBOD = 30 mg/L TSS = 90 mg/L NH ₃ = 2 mg/L Flow = 0.4 MGD	2005 = 0.24 2010 = 0.27 2020 = 0.33 2030 = 0.38 2040 = 0.44 2050 = 0.52	Bar screen, grit removal, oxidation ditch w/o RAS followed by three stabilization ponds, and a nitrifying bio-tower. Disinfection not required. Sludge is periodically dredged from the ponds and disposed offsite.	Expansion required between 2030 and 2040. Expand headworks and pumping capacity. Add aerators to stabilization ponds and upgrade the bio-tower. Add chlorine disinfection and drying beds.	Maintain pond volume for N removal and add alum for P removal.	Add denitrifying filter to meet TN limit, carbon source for denitrification and additional alum for lower P limit.
			Project cost: \$635,242 Additional Annual O&M cost: \$98,700	Increase in cost from Level I: \$127,512 Increase in Annual O&M cost over Level I: \$5,355	Increase in cost from Levels I and II: \$835,783 Increase in Annual O&M cost over Levels I and II: \$31,290
TERRELL CBOD = 10 mg/L TSS = 15 mg/L NH ₃ = 6.0 mg/L Flow = 4.5 MGD	2005 = 3.23 2010 = 3.42 2020 = 4.19 2030 = 4.87 2040 = 5.32 2050 = 5.76 ³	Flow equalization, bar screen, grit removal, primary clarification, two-stage trickling filter, final clarification, with chlorination and dechlorination. Portable belt press solids dewatering.	Expansion required between 2020 and 2030. Add additional headworks with screens and grit removal. Add influent pump capacity, Sequencing Batch Reactors and sand filters. Expand chlorine contact basin and add gravity belt thickener.	Add denitrifying filters to existing treatment train. Optimize SBRs for denitrification. Alum addition for P removal.	Carbon source for denitrification and additional alum for lower P limit.
			Project cost: \$7,416,562 Additional Annual O&M cost: \$101,115	Increase in cost from Level I: \$3,192,437 Increase in Annual O&M cost over Level I: \$15,855	Increase in cost from Levels I and II: \$146,059 Increase in Annual O&M cost over Levels I and II: \$178,500

Table ES-1
Tarrant Regional Water District
Cedar Creek Lake
Wastewater Plant Dischargers Assessment

Treatment Plant & Permit Limits	Flow Projections ¹ (MGD) <u>Present</u> BOD ₅ = 30 mg/L TSS = 90 mg/L Flow = 0.51 MGD <u>Planned</u> CBOD = 10 mg/L TSS = 15 mg/L NH ₃ = 3 mg/L Flow = 0.80 MGD	Existing Process Description	Facility Needs for 2050 and Probable Construction and O&M Costs		
			Level I Existing Permit Limits	Level II ² TN = 10 mg/L TP = 1 mg/L	Level III ³ TN = 5 mg/L TP = 0.5 mg/L
WILLS POINT		<p>Two new stabilization ponds are currently under expansion. Bar screen, two facultative lagoons, three stabilization ponds exist. Disinfection not required. Sludge is periodically dredged from the ponds and disposed offsite.</p> <p>Once current expansion is complete, facility is capable of treating the projected flows through the year 2050.</p>	<p>Operate ponds for denitrification, and dredge ponds to maintain volume. Add earthen pond divider. Alum addition for P removal.</p> <p>Project cost: \$0</p> <p>Additional Annual O&M cost: \$0</p>	<p>Increase in cost from Levels I and II: \$835,783</p> <p>Increase in Annual O&M cost over Level I: \$4,935</p>	<p>Denitrifying filter to meet TN limit., with possible additional carbon source. Additional alum for lower P limit.</p>

¹ Flow projections based on TWDB population projections and 100 gpcd unless otherwise noted.

² Level II O&M costs are in addition to current operating costs.

³ Level III capital costs are in addition to Level I costs. Level III capital costs are in addition to Level I and Level II costs.

The initial opinion of costs presented herein represents a Class 5 estimate based upon ANSI Standard Reference Z94.2-1989 and AACE International Recommended Practice No. 17R-97.

Table ES-2: Comparison of Costs for Each Plant's Expansion or Upgrade

City or Facility	Costs Associated with Upgrades		
	Level I	Level II	Level III
Athens	\$3,428,914	\$893,743	\$76,507
Cherokee Shores	\$0	\$592,351	\$64,915
East Cedar Creek	\$4,915,008	\$127,512	\$912,290
Eustace	\$217,930	\$51,005	\$549,461
Kaufman	\$2,088,878	\$1,659,974	\$92,736
Kemp	\$0	\$51,005	\$537,869
Mabank	\$635,242	\$127,512	\$835,783
Terrell	\$7,416,562	\$3,192,437	\$146,059
Wills Point	\$0	\$231,840	\$835,783
Total Costs	\$18,702,534	\$6,927,379	\$4,051,403

Table ES-3: Nutrient Loads in Lbs/day and Tons/year for Each Plant at Each Permit Level

City or Facility	Loads Associated with 2050 Flows (lbs/day)						Loads Associated with 2050 Flows (tons/yr)					
	Total Nitrogen			Total Phosphorous			Total Nitrogen			Total Phosphorous		
	Level I	Level II	Level III	Level I	Level II	Level III	Level I	Level II	Level III	Level I	Level II	Level III
Athens	129	95	48	27	10	5	23	17	9	5	2	1
Cherokee Shores	57	15	8	6	2	0.8	10	3	1	1	0.27	0.14
East Cedar Creek	312	133	67	29	13	7	57	24	12	5	2	1
Eustace	25	11	5	5	1	0.5	5	2	1	1	0.19	0.10
Kaufman	186	138	69	39	14	7	34	25	13	7	3	1
Kemp	14	9	5	3	0.9	0.5	2	2	1	1	0.17	0.09
Mabank	51	43	22	17	4	2	9	8	4	3	1	0.40
Terrell	947	480	240	194	48	24	173	88	44	35	9	4
Wills Point	51	43	21	11	4	2	9	8	4	2	1	0.39
Level II Loads	1,771	967	484	332	97	48	323	177	88	61	18	9
Load Reduction from Level I to Level II	804			235			147			43		
Load Reduction from Level II to Level III				48						88		9

* Level I loads calculated from average TN and TP as reported in each chapter's Table 3

SECTION I

CITY OF ATHENS NORTH WASTEWATER TREATMENT PLANT

SECTION I

CITY OF ATHENS NORTH WASTEWATER TREATMENT PLANT

INTRODUCTION

The Athens North WWTP discharges into Caney Creek and then flows into the Cedar Creek Reservoir in Segment 0818 of the Trinity River Basin. Sources of information for the evaluation included: a site visit, an interview with the plant personnel, review of the existing plans and historical reports, data collected by plant personnel for the District, and the data acquired through Texas Commission on Environmental Quality (TCEQ). This section is organized as follows:

- Description of the existing TPDES permit conditions and projected future limits
- Historical effluent data
- Brief description of the treatment process
- Population growth and flow projections
- Facility needs and costs to meet Level I effluent limits through 2050
- Facility needs and costs to meet Level II effluent limits through 2050
- Facility needs and costs to meet Level III effluent limits through 2050

EXISTING PERMIT CONDITIONS

The City of Athens operates two wastewater treatment plants. One is referred to as the North plant and the other as the West. The West plant, which does not discharge into the Cedar Creek watershed, is permitted for a flow of 1.32 million gallons per day (MGD). The West plant operates under TPDES Permit No. 10143-001. Since the West plant does not discharge into the Cedar Creek sewershed, an evaluation of the WWTP is not presented in this study.

The North plant discharges into the Cedar Creek sewershed and has a rated capacity of 1.027 MGD. The plant presently treats about 40% of the City's wastewater. The North wastewater treatment plant operates under TPDES Permit No. 10143-001-A with the current permit limits for the North plant as shown in Table I-1; limits shown are the current permit limits, which may have changed throughout the course of historical data collection and report writing.

Table I-1
City of Athens North WWTP
Current TPDES Permit Conditions
TPDES Permit 10143-001-A

Parameters	Permit Limits March - November	Permit Limits December - February
Carbonaceous Biochemical Oxygen Demand (CBOD)	10 mg/L	20 mg/L
Total Suspended Solids (TSS)	15 mg/L	20 mg/L
Ammonia Nitrogen (NH ₃)	2 mg/L	3 mg/L
Annual Average Flow	1.027 MGD	1.027 MGD

The existing permit conditions are referred to as Level I. The next level of treatment, referred to as Level II, includes reduction of phosphorus to 1 mg/L and total nitrogen to 10 mg/L. The most stringent level of treatment considered, referred to as Level III, includes reduction of phosphorus to 0.5 mg/L and

total nitrogen to 5 mg/L. The assessment of facility needs is performed at all three levels for flows projected for 2050.

HISTORICAL PLANT FLOWS AND EFFLUENT QUALITY

A summary of the historical flows and effluent quality for the City of Athens' North plant are presented in Table I-2; all data in this table were derived from the plant Discharge Monitoring Reports (DMRs). The flow to the plant is currently about 40% of its rated capacity.

Table I-2
City of Athens North WWTP
Historical Plant Flows and Effluent Quality

Year	Average Daily Flow	Effluent		
		CBOD	TSS	NH ₃ -N
		(MGD)	(mg/L)	(mg/L)
1999	0.42	9.1	7.9	3.7
2000	0.50	10.5	8.3	4.4
2001	0.45	7.8	9.8	3.1
2002	0.45	3.4	7.5	1.9
2003	0.42	3.1	8.7	0.6

The North plant typically meets its permit limits for CBOD and TSS. It has met its ammonia limit for the most recent years of data collection. At the request of the Tarrant Regional Water District, the city of Athens collected 12 months of effluent nutrient data. The data indicate that the plant typically meets its ammonia limit with occasional excursions. Operational and facility changes will be required to meet Level II and III effluent goals. A summary of the nutrient data is presented in Table I-3; all data may be found in Appendix A.

Table I-3
City of Athens WWTP
Special Nutrient Testing for Tarrant Regional Water District

	NH ₃ -N	NO _x	IP	Org P	Org N	TN	TP
No. of Data Points	49	49	49	49	49	49	49
Average (mg/L)	1.68	10.91	1.72	1.16	1.34	13.53	2.85
Minimum (mg/L)	0.20	0.19	0.84	0.00	0.00	0.55	1.40
Maximum (mg/L)	20.00	27.00	3.70	2.40	8.05	27.52	4.40

Figures I-1 and I-2 illustrate the seasonal variability of total nitrogen and phosphorous loads.

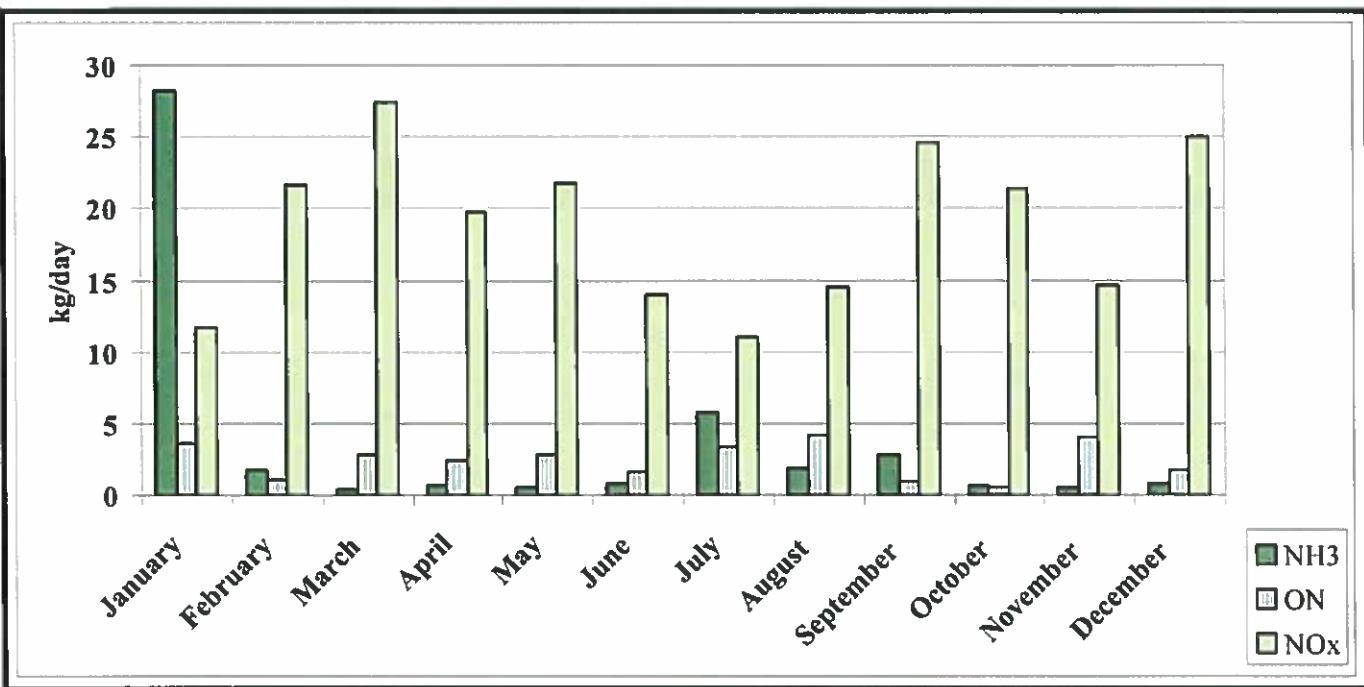


Figure I-1: Athens Effluent Total Nitrogen Monthly Average Loads

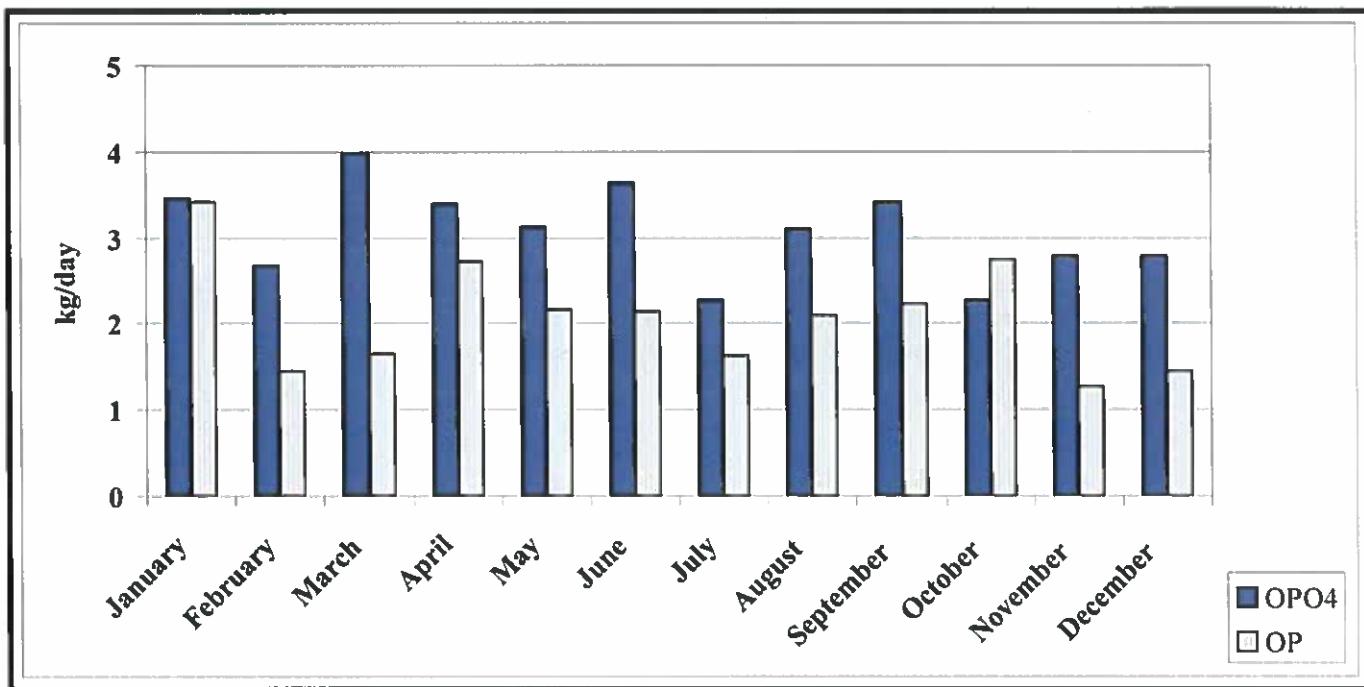


Figure I-2: Athens Effluent Total Phosphorous Monthly Average Loads

PROCESS OVERVIEW

The City's North plant is a conventional wastewater treatment plant utilizing an Imhoff tank for primary settling, trickling filters, aeration basins, and final clarification for treatment. See Figure I-3 for the plant's process schematic. Table I-4 lists the existing treatment units and their dimensions.

**Table I-4
City of Athens North WWTP
Existing Treatment Facilities**

Treatment Units	Dimensions/Size
Imhoff Tank No. 1 Volume	517,000 gallons
Imhoff Tank No. 2 Volume	96,900 gallons
Trickling Filters Nos. 1 and 2	60-ft diameter, 12-ft SWD
Aeration Basin	20-ft x 10-ft, 15-ft SWD 22,400 gallons
Final Clarifiers Nos. 1 and 2	40-ft diameter, 12-ft SWD 112,800 gallons
Chlorine Contact Basin Volume	92,000 gallons

Wastewater from the City of Athens enters the treatment facility through an 18-inch diameter gravity sewer line. The raw wastewater is passed through a manually cleaned bar screen to remove larger solids after which the grit is removed. The raw wastewater is pumped into Imhoff tanks. Settling occurs in the top compartment and degradation of the sludge under absence of oxygen occurs in the lower unheated compartment. The supernatant from the Imhoff tanks is routed to a trickling filter for further treatment of organic matter present in the wastewater; Trickling Filter No. 1 utilizes rock while No. 2 employs a plastic media. The trickling filter effluent is aerated in an aeration basin for nitrification. From the aeration basin, flow is sent to secondary clarifiers to separate the liquid and the biomass. The supernatant from the secondary clarifiers is then disinfected with chlorine solution and discharged into a creek.

Primary sludge and biological sludge are periodically wasted to the sludge drying beds. Decant water from the sludge drying bed is mixed with the raw wastewater prior to the Imhoff tanks. Dried sludge is periodically removed from the drying beds and disposed of off-site.

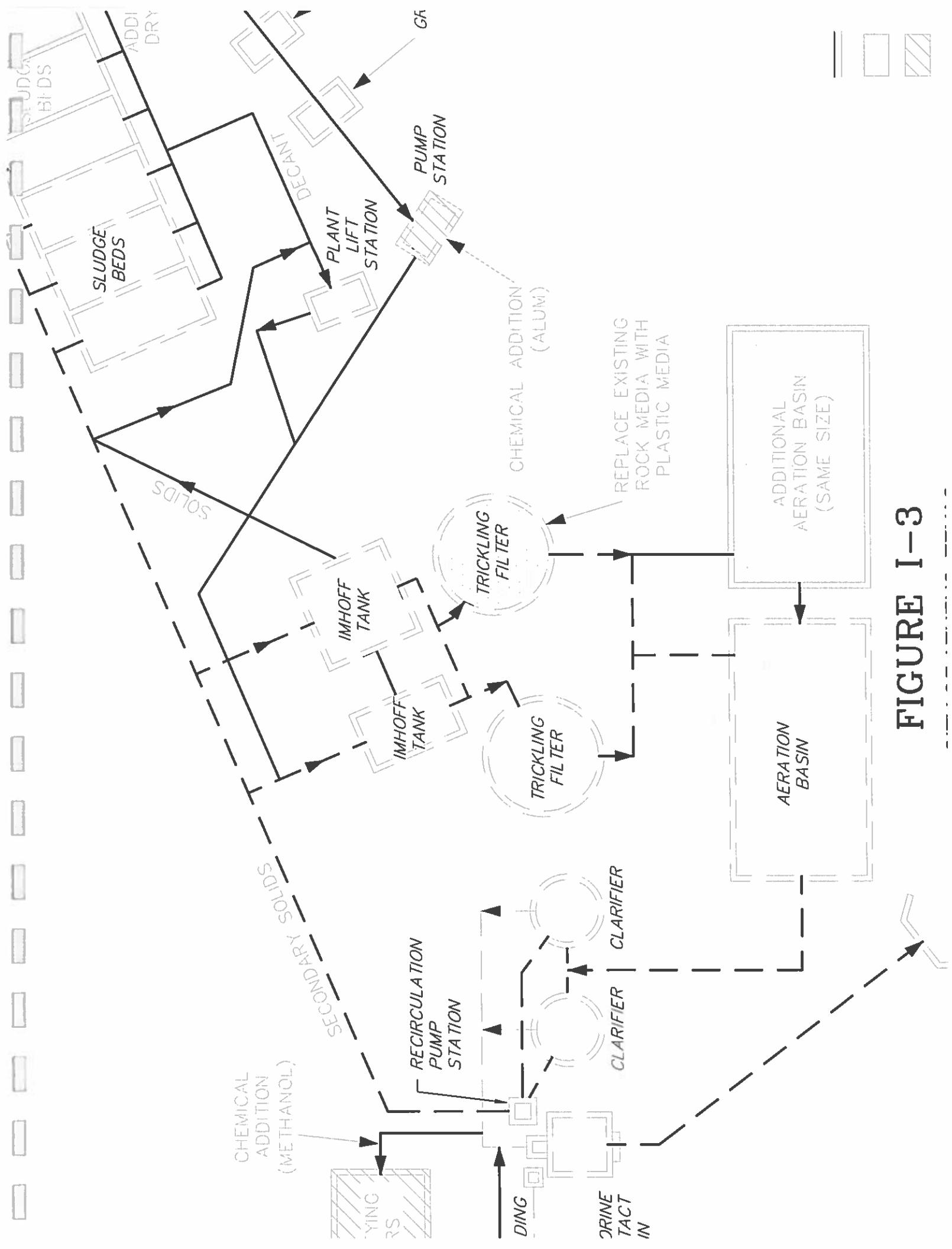


FIGURE I-3

FUTURE FLOW PROJECTIONS

In the year 2000, the population of the City of Athens was 11,297. Projected populations and flows through the year 2050 are listed in Table I-5.

**Table I-5
City of Athens
Population and Flow Projections**

Year	2005	2010	2020	2030	2040	2050
Population ¹	12,390	13,588	16,343	19,657	23,643	28,438
Projected Flow (MGD) ² North WWTP	0.50	0.54	0.65	0.78	0.94	1.14
Projected Flow (MGD) ² West WWTP	0.74	0.82	0.98	1.18	1.42	1.7

¹ Texas Water Development Board 2006 Regional Water Plan

² Average Flow = assumed 100 gal/capita/day (Figure 1, 30 TAC 317.4 (a))

Table I-5 flow projections are based on the assumption that the flow split between the North (40%) and West (60%) WWTPs will continue as it is currently. The projected 2050 flow for the North plant is 1.14 MGD. The projected flow will exceed the rated capacity of the North plant around the year 2045.

FACILITY NEEDS EVALUATION

This section describes the facilities identified for the plant to meet each of the three levels of treatment for flows through the year 2050. It should be noted that many of the existing facilities will be beyond their useful lives by the year 2050. The identification of the facility needs does not include the replacement of worn out equipment or treatment units.

Level I Facility Needs for 2050 Wastewater Flows

Level I treatment requirements are identified based on continuing to meet its existing effluent limits. As mentioned above, the plant's current permitted daily average flow is 1.027 MGD. Around the year 2045, the projected flow will exceed the current plant capacity, and there will be a need to expand the plant.

To treat year 2050 projected flows, the existing plant needs to be expanded to 1.14 MGD. An additional capacity of 0.109 MGD is required. The North WWTP needs to expand its influent pumping system to handle the higher flows. One of the existing trickling filters has rock media that can be replaced with plastic media to expand its treatment capability. An aeration basin of similar size to the existing basin needs to be added to achieve treatment capacity to treat year 2050 projected flows. Three additional sludge-drying beds of existing bed size would provide sufficient capacity through the year 2050.

The capital costs involved to improve the City's plant to treat the year 2050 flow are listed in Table I-6, as well as additional annual operation costs.

Table I-6
City of Athens North WWTP
Facility Needs to Meet Level I Effluent Requirements and 2050 Flows

Item Description		Cost
Influent Pumping		\$139,000
Replace Rock Media with Plastic Media		\$94,000
New Aeration Basin		\$1,186,000
Three Additional Drying Beds		\$60,000
	Sub-Total	\$1,479,000
Site Work and Yard Piping	20%	\$295,800
	Sub-Total	\$1,774,800
Electrical and Instrumentation	20%	\$354,960
	Sub-Total	\$2,129,760
Contingencies	25%	\$532,440
Contractor OHP	15%	\$319,464
	Total Construction Cost	\$2,981,664
Engineering	10%	\$298,166
Permitting	5%	\$149,083
	Total Project Cost	\$3,428,914
Power	75 hp and \$0.12 per kW-hr	\$60,000
Chemical		\$0
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5% \$3,000
	Total O&M Cost per Year	\$63,000

Level II Facility Needs for 2050 Wastewater Flows

The Level II effluent set facility needs are based on reducing the effluent phosphorus concentration to 1 mg/L, total nitrogen to 10 mg/L, CBOD to 10 mg/L, TSS to 15 mg/L, and ammonia nitrogen to 2 mg/L. As total phosphorus average concentrations measured during the nutrient testing were greater than 1 mg/L, this evaluation assumes phosphorus effluent concentration reduction is necessary. To consistently achieve phosphorus concentrations of less than 1 mg/L, the addition of aluminum sulfate (alum) is recommended. Alum solution will be fed at the influent pump station to achieve required mixing of the alum solution. The use of alum may retard drying of the sludge on the drying beds. It may be necessary to add more drying beds or modify the existing ones to accommodate alum sludge.

A denitrifying filter will be added to meet the effluent total nitrogen limits. An alternative to the denitrifying filter would involve constructing anoxic zones in the aeration basin and recirculating nitrified effluent to the anoxic zone. This alternative should be evaluated during preliminary design.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level II effluent requirements are listed in Table I-7.

Table I-7
City of Athens North WWTP
Facility Needs to Meet Level II Effluent Requirements and 2050 Flows

Item Description		Cost
Denitrification Filter		\$330,500
Chemical Feed System for Alum		\$55,000
	Sub-Total	\$385,500
Site Work and Yard Piping	20%	\$77,100
	Sub-Total	\$462,600
Electrical and Instrumentation	20%	\$92,520
	Sub-Total	\$555,120
Contingencies	25%	\$138,780
Contractor OHP	15%	\$83,268
	Total Construction Cost	\$777,168
Engineering	10%	\$77,717
Permitting	5%	\$38,858
	Total Project Cost	\$893,743
Power	25 hp and \$0.12 per kW-hr	\$20,000
Chemical		\$1,800
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5%
		\$1,090
	Total O&M Cost per Year	\$22,890

Level III Facility Needs for 2050 Wastewater Flows

The Level III effluent set facility needs are based on reducing the effluent phosphorus concentration to 0.5 mg/L, total nitrogen to 5 mg/L, CBOD to 10 mg/L, TSS to 15 mg/L, and ammonia nitrogen to 2 mg/L. To achieve the Level III concentrations of phosphorus, higher dosages of alum will be required. Additional carbon may be required at the denitrifying filters to reduce total nitrogen levels to less than 5 mg/L.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level III effluent requirements are listed in Table I-8.

Table I-8
City of Athens North WWTP
Facility Needs to Meet Level III Effluent Requirements and 2050 Flows

Item Description		Cost
Chemical Feed System for Methanol		\$33,000
	Sub-Total	\$33,000
Site Work and Yard Piping	20%	\$6,600
	Sub-Total	\$39,600
Electrical and Instrumentation	20%	\$7,920
	Sub-Total	\$47,520
Contingencies	25%	\$11,880
Contractor OHP	15%	\$7,128
	Total Construction Cost	\$66,528
Engineering	10%	\$6,653
Permitting	5%	\$3,326
	Total Project Cost	\$76,507
Power		\$0
Chemical		\$36,700
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5%
		\$1,835
	Total O&M Cost per Year	\$38,535

SECTION II

CHEROKEE SHORES WASTEWATER TREATMENT PLANT

SECTION II

CHEROKEE SHORES WASTEWATER TREATMENT PLANT

INTRODUCTION

The Cherokee Shores wastewater treatment facility (WWTF) discharges into the Cedar Creek Reservoir in Segment 0818 of the Trinity River Basin. Sources of information for the evaluation included: a site visit, an interview with the plant personnel, review of the existing plans and historical reports, data collected by plant personnel for the District, and the data acquired through Texas Commission on Environmental Quality (TCEQ). This section is organized as follows:

- Description of the existing TPDES permit conditions
- Historical effluent data
- Brief description of the treatment process
- Population growth and flow projections
- Facility needs and costs to meet Level I effluent limits through 2050
- Facility needs and costs to meet Level II effluent limits through 2050
- Facility needs and costs to meet Level III effluent limits through 2050

EXISTING PERMIT CONDITIONS

Tecon Water Company, L.P., owns and operates the Cherokee Shores wastewater treatment facility. The wastewater treatment plant is operated under TPDES Permit No. 13879-001 and has a rated capacity of 0.15 million gallons per day (MGD). The permit limits are shown in Table II-1; limits shown are final permit limits, which may have changed throughout the course of historical data collection and report writing.

Table II-1
Cherokee Shores WWTF
Current TPDES Permit Conditions
TPDES Permit No. 13879-001

Parameters	Permit Limits
BOD ₅	10 mg/L
Total Suspended Solids (TSS)	15 mg/L
Ammonia Nitrogen (NH ₃)	5 mg/L
Total Nitrogen	20 mg/L
Total Phosphorus	2 mg/L
Flow	0.15 MGD

The existing permit conditions are referred to as Level I. The next level of treatment, referred to as Level II, includes reduction of phosphorus to 1 mg/L and total nitrogen to 10 mg/L. The most stringent level of treatment considered, referred to as Level III, includes reduction of phosphorus to 0.5 mg/L and total nitrogen to 5 mg/L. The assessment of facility needs is performed at all three levels for flows projected for 2050.

HISTORICAL PLANT FLOWS AND EFFLUENT QUALITY

A summary of the historical flows and effluent quality for Cherokee Shores wastewater treatment facility are presented in Table II-2; all data in this table were derived from the plant Discharge Monitoring Reports (DMRs). As shown, the flow to the plant is currently about one-half of its rated capacity.

Table II-2
Cherokee Shores WWTF
Historical Plant Flows and Effluent Quality

Year	Average Daily Flow (MGD)	Effluent	
		BOD ₅ (mg/L)	TSS (mg/L)
1999	0.06	9.7	8.3
2000	0.05	11.3	11.5
2001	0.06	18.4	25.3
2002	0.08	9.4	20.4
2003	0.09	8.4	18.3
ND: No Data			

Historically, the plant has not met its effluent permit limits for TSS, and has had some difficulty in consistently meeting the BOD₅ limit. The plant has applied for and received a permit for expansion, the limits of which are those in Table II-1, and these final limits after expansion will be considered Level I for the purposes of this report. During the course of writing this report, the plant was subject to no permit limits for total nitrogen, phosphorous, or ammonia and the permitted flow was 0.075 MGD.

At the request of the Tarrant Regional Water District, Cherokee Shores collected 12 months of effluent nutrient data. During the 12 months of data collection, the plant's average nutrient concentrations were above the effluent limits for the final permit. A review of operations should be done to determine the reason for non-compliance. Operational and facility changes will be required to meet Level II and Level III effluent goals. A summary of the data is presented in Table II-3; all data may be found in Appendix A.

Table II-3
Cherokee Shores WWTF
Special Nutrient Testing for Tarrant Regional Water District

	NH ₃ -N	NO _x	IP	Org P	Org N	TN	TP
No. of Data Points	50	50	50	50	50	50	50
Average (mg/L)	30.6	1.8	3.4	0.9	5.6	37.8	4.3
Minimum (mg/L)	5.5	0.4	0.2	0.0	0.0	18.4	0.5
Maximum (mg/L)	42.0	27.0	16.5	3.4	30.3	50.7	18.6

Figures II-1 and II-2 illustrate the seasonal variability of total nitrogen and phosphorous loads.

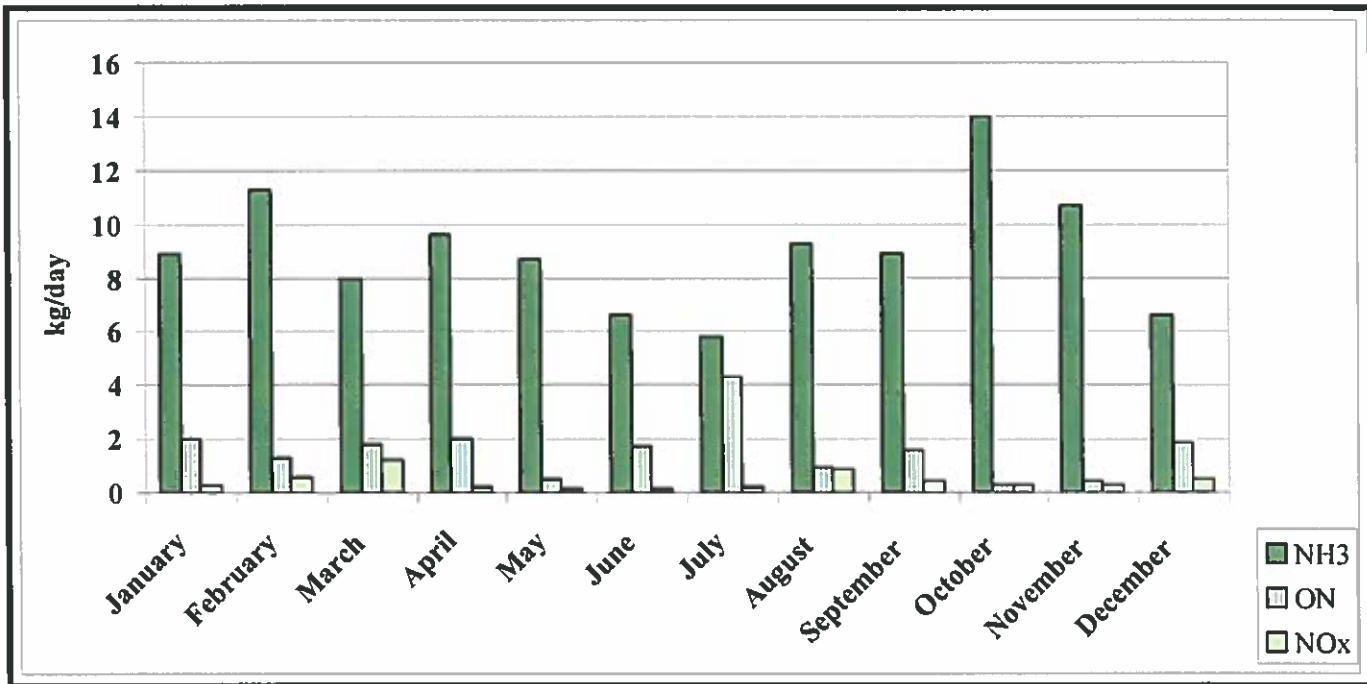


Figure II-1: Cherokee Shores Effluent Total Nitrogen Monthly Average Loads

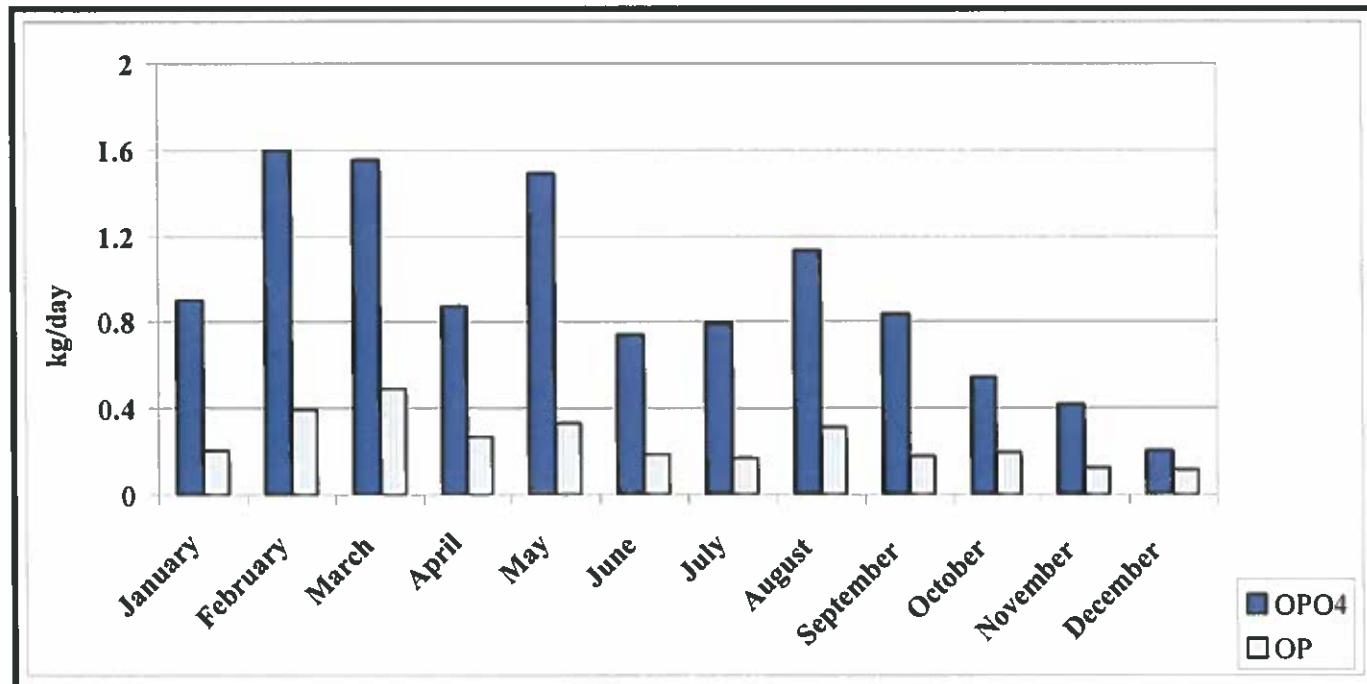


Figure II-2: Cherokee Shores Effluent Total Phosphorous Monthly Average Loads

PROCESS OVERVIEW

The Cherokee Shores WWTF is a conventional wastewater treatment plant utilizing flow equalization, aeration basins, final clarification, filtration, and disinfection for treatment. The City's WWTF rated treatment capacity will be 0.15 MGD, but at the time of this report the plant was operating under a 0.075 MGD limit. See Figure II-3 for a schematic of the facilities processes. Table II-4 lists the existing treatment units and their current sizes.

Table II-4
Cherokee Shores WWTF
Existing Treatment Facilities

Treatment Units	Dimensions/Sizes
Aeration Basin	72-ft x 12-ft, 8-ft SWD 51,700 gallons
Circular Clarifier	20-ft diameter, 6-ft SWD 14,000 gallons
Filter Surface Area	80 square feet (each)
Chlorine Contact Basin Volume	7,200 gallons
Equalization Basin Volume	2,350 gallons
Aerobic Digester Volume	16,150 gallons

Influent wastewater is either routed to the aeration basin or to a flow equalization tank. The primary purpose of the equalization tank is to hold peak flows for short time periods so as not to exceed the capacity of the other treatment units. The equalization tank can also be used to equalize flow with varying characteristics. The wastewater is treated in an aeration basin where microorganisms stabilize the organics present in the wastewater. Flow from the aeration basin is settled in the clarifier to separate out the biomass. The clarifier overflow is then passed through a sand filtration system and disinfected with chlorine solution prior to discharge.

A portion of settled sludge is returned to the aeration basin as return activated sludge and the remainder is sent to an aerobic digester. Digested sludge is pumped from the digester and disposed off-site.



*Tarrant Regional Water District
Watershed Project*

*Cedar Creek Reservoir
Wastewater Treatment Facilities Report*

*Evaluating Water Quality Management Practices for
Reservoirs in North Central Texas*

*Revised
May 5, 2008*





Tarrant Regional Water District Watershed Project

Cedar Creek Reservoir Wastewater Treatment Facilities Report

Table of Contents

List of Tables	iv
List of Figures.....	vii
ES. Executive Summary	ES-1
I. City of Athens	I-1
Introduction	I-1
Existing Permit Conditions	I-1
Historical Plant Flows and Effluent Quality	I-2
Process Overview	I-3
Future Flow Projections	I-6
Facility Needs Evaluation	I-6
Level I Facility Needs for 2050 Wastewater Flows	I-6
Level II Facility Needs for 2050 Wastewater Flows.....	I-7
Level III Facility Needs for 2050 Wastewater Flows	I-8
II. Cherokee Shores	II-1
Introduction	II-1
Existing Permit Conditions	II-1
Historical Plant Flows and Effluent Quality	II-2
Process Overview.....	II-4
Future Flow Projections	II-6
Facility Needs Evaluation	II-6
Level I Facility Needs for 2050 Wastewater Flows	II-6
Level II Facility Needs for 2050 Wastewater Flows.....	II-6
Level III Facility Needs for 2050 Wastewater Flows	II-7
III. East Cedar Creek FWSD North WWTP	III-1
Introduction	III-1
Existing Permit Conditions	III-1
Historical Plant Flows and Effluent Quality	III-2
Process Overview.....	III-4
Future Flow Projections	III-6
Facility Needs Evaluation	III-6
Level I Facility Needs for 2050 Wastewater Flows	III-6
Level II Facility Needs for 2050 Wastewater Flows.....	III-7
Level III Facility Needs for 2050 Wastewater Flows	III-8



IV.	City of Eustace	IV-I
	Introduction	IV-1
	Existing Permit Conditions	IV-1
	Historical Plant Flows and Effluent Quality	IV-2
	Process Overview	IV-4
	Future Flow Projections	IV-6
	Facility Needs Evaluation	IV-6
	Level I Facility Needs for 2050 Wastewater Flows	IV-6
	Level II Facility Needs for 2050 Wastewater Flows.....	IV-7
	Level III Facility Needs for 2050 Wastewater Flows	IV-8
V.	City of Kaufman	V-1
	Introduction	V-1
	Existing Permit Conditions	V-1
	Historical Plant Flows and Effluent Quality	V-2
	Process Overview	V-4
	Future Flow Projections	V-6
	Facility Needs Evaluation	V-6
	Level I Facility Needs for 2050 Wastewater Flows	V-6
	Level II Facility Needs for 2050 Wastewater Flows.....	V-7
	Level III Facility Needs for 2050 Wastewater Flows	V-8
VI.	City of Kemp	VI-I
	Introduction	VI-1
	Existing Permit Conditions	VI-1
	Historical Plant Flows and Effluent Quality	VI-2
	Process Overview	VI-4
	Future Flow Projections	VI-6
	Facility Needs Evaluation	VI-6
	Level I Facility Needs for 2050 Wastewater Flows	VI-6
	Level II Facility Needs for 2050 Wastewater Flows.....	VI-6
	Level III Facility Needs for 2050 Wastewater Flows	VI-7
VII.	City of Mabank	VII-1
	Introduction	VII-1
	Existing Permit Conditions	VII-1
	Historical Plant Flows and Effluent Quality	VII-2
	Process Overview	VII-4
	Future Flow Projections	VII-6
	Facility Needs Evaluation	VII-6
	Level I Facility Needs for 2050 Wastewater Flows	VII-6
	Level II Facility Needs for 2050 Wastewater Flows.....	VII-7
	Level III Facility Needs for 2050 Wastewater Flows	VII-8



VIII.	City of Terrell King's Creek WWTP	VIII-1
	Introduction	VIII-1
	Existing Permit Conditions	VIII-1
	Historical Plant Flows and Effluent Quality	VIII-2
	Process Overview	VIII-4
	Future Flow Projections	VIII-6
	Facility Needs Evaluation	VIII-6
	Level I Facility Needs for 2050 Wastewater Flows	VIII-6
	Level II Facility Needs for 2050 Wastewater Flows.....	VIII-7
	Level III Facility Needs for 2050 Wastewater Flows	VIII-8
IX.	City of Wills Point	IX-1
	Introduction	IX-1
	Existing Permit Conditions	IX-1
	Historical Plant Flows and Effluent Quality	IX-2
	Process Overview.....	IX-4
	Future Flow Projections	IX-6
	Facility Needs Evaluation	IX-6
	Level I Facility Needs for 2050 Wastewater Flows	IX-6
	Level II Facility Needs for 2050 Wastewater Flows.....	IX-6
	Level III Facility Needs for 2050 Wastewater Flows	IX-7

Appendices

City of Athens Nutrient Data	A-1
Cherokee Shores Nutrient Data.....	A-2
East Cedar Creek FWSD Nutrient Data.....	A-3
City of Eustace Nutrient Data.....	A-4
City of Kaufman Nutrient Data.....	A-5
City of Kemp Nutrient Data.....	A-6
City of Mabank Nutrient Data.....	A-7
City of Terrell King's Creek WWTP Nutrient Data	A-8
City of Wills Point Nutrient Data	A-9



Tarrant Regional Water District Watershed Project

Cedar Creek Reservoir Wastewater Treatment Facilities Report

List of Tables

Table ES-1:	Tarrant Regional Water District Cedar Creek Lake Wastewater Plant Dischargers Assessment	ES-3
Table ES-2:	Comparison of Costs for Each Plant's Expansion or Upgrade.....	ES-9
Table ES-3:	Nutrient Loads in Lbs/day and Tons/year for Each Plant at Each Permit Level	ES-9
Table I-1:	City of Athens North WWTP Current TPDES Permit Conditions	I-1
Table I-2:	City of Athens North WWTP Historical Plant Flows and Effluent Quality	I-2
Table I-3:	City of Athens WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	I-2
Table I-4:	City of Athens North WWTP Existing Treatment Facilities.....	I-4
Table I-5:	City of Athens Population and Flow Projections	I-6
Table I-6:	City of Athens North WWTP Facility Needs to Meet Level I Effluent Requirements and 2050 Flows	I-7
Table I-7:	City of Athens North WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	I-8
Table I-8:	City of Athens North WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	I-9
Table II-1:	Cherokee Shores WWTF Current TPDES Permit Conditions	II-1
Table II-2:	Cherokee Shores WWTF Historical Plant Flows and Effluent Quality	II-2
Table II-3:	Cherokee Shores WWTF Special Nutrient Testing by Texas A&M Agricultural Extension	II-2
Table II-4:	Cherokee Shores WWTF Existing Treatment Facilities	II-4
Table II-5:	Cherokee Shores WWTF Population and Flow Projections	II-6
Table II-6:	Cherokee Shores WWTF Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	II-7
Table II-7:	Cherokee Shores WWTF Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	II-8
Table III-1:	ECCFWSD North WWTP Current TPDES Permit Conditions	III-1
Table III-2:	ECCFWSD North WWTP Historical Plant Flows and Effluent Quality	III-2
Table III-3:	ECCFWSD WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	III-2
Table III-4:	ECCFWSD North WWTP Existing Treatment Facilities	III-4
Table III-5:	ECCFWSD Population and Flow Projections	III-6
Table III-6:	ECCFWSD North WWTP Facility Needs to Meet Level I Effluent Requirements and 2050 Flows	III-7
Table III-7:	ECCFWSD North WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	III-8



Tarrant Regional Water District Watershed Project

Cedar Creek Reservoir Wastewater Treatment Facilities Report

List of Tables, continued

Table III-8:	ECCFWSD North WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	III-9
Table IV-1:	City of Eustace WWTP Current TPDES Permit Conditions.....	IV-1
Table IV-2:	City of Eustace WWTP Historical Plant Flows and Effluent Quality.....	IV-2
Table IV-3:	City of Eustace WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	IV-2
Table IV-4:	City of Eustace WWTP Existing Treatment Facilities.....	IV-4
Table IV-5:	City of Eustace Population and Flow Projections	IV-6
Table IV-6:	City of Eustace WWTP Facility Needs to Meet Level I Effluent Requirements and 2050 Flows	IV-7
Table IV-7:	City of Eustace WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	IV-8
Table IV-8:	City of Eustace WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	IV-9
Table V-1:	City of Kaufman WWTP Current TPDES Permit Conditions	V-1
Table V-2:	City of Kaufman WWTP Historical Plant Flows and Effluent Quality	V-2
Table V-3:	City of Kaufman WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	V-2
Table V-4:	City of Kaufman WWTP Existing Treatment Facilities	V-4
Table V-5:	City of Kaufman WWTP Population and Flow Projections.....	V-6
Table V-6:	City of Kaufman WWTP Facility Needs to Meet Level I Effluent Requirements and 2050 Flows	V-7
Table V-7:	City of Kaufman WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	V-8
Table V-8:	City of Kaufman WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	V-9
Table VI-1:	City of Kemp WWTP Current TPDES Permit Conditions	VI-1
Table VI-2:	City of Kemp WWTP Historical Plant Flows and Effluent Quality	VI-2
Table VI-3:	City of Kemp WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	VI-2
Table VI-4:	City of Kemp WWTP Existing Treatment Facilities	VI-4
Table VI-5:	City of Kemp Population and Flow Projections.....	VI-6
Table VI-6:	City of Kemp WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	VI-7
Table VI-7:	City of Kemp WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	VI-8



Tarrant Regional Water District Watershed Project

Cedar Creek Reservoir Wastewater Treatment Facilities Report

List of Tables, continued

Table VII-1:	City of Mabank WWTP Current TPDES Permit Conditions.....	VII-1
Table VII-2:	City of Mabank WWTP Historical Plant Flows and Effluent Quality	VII-2
Table VII-3:	City of Mabank WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	VII-2
Table VII-4:	City of Mabank WWTP Existing Treatment Facilities	VII-4
Table VII-5:	City of Mabank Population and Flow Projections	VII-6
Table VII-6:	City of Mabank WWTP Facility Needs to Meet Level I Effluent Requirements and 2050 Flows	VII-7
Table VII-7:	City of Mabank WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	VII-8
Table VII-8:	City of Mabank WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	VII-9
Table VIII-1:	City of Terrell King's Creek WWTP Current Permit Conditions	VIII-1
Table VIII-2:	City of Terrell King's Creek WWTP Historical Plant Flows and Effluent Quality	VIII-2
Table VIII-3:	City of Terrell King's Creek WWTP Special Nutrient Testing by Texas A&M Agricultural Extension	VIII-2
Table VIII-4:	City of Terrell King's Creek WWTP Existing Treatment Facilities	VIII-4
Table VIII-5:	City of Terrell Population and Flow Projections.....	VIII-6
Table VIII-6:	City of Terrell King's Creek WWTP Facility Needs to Meet Level I Effluent Requirements and 2050 Flows	VIII-7
Table VIII-7:	City of Terrell King's Creek WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	VIII-8
Table VIII-8:	City of Terrell King's Creek WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	VIII-9
Table IX-1:	City of Wills Point WWTP Current TPDES Permit Conditions	IX-1
Table IX-2:	City of Wills Point WWTP Historical Plant Flows and Effluent Quality	IX-2
Table IX-3:	City of Wills Point WWTP Special Nutrient Testing by Texas A&M Agricultural Extension.....	IX-2
Table IX-4A:	City of Wills Point WWTP Existing Treatment Facilities	IX-4
Table IX-4B:	City of Wills Point WWTP Planned Treatment Facilities.....	IX-4
Table IX-5:	City of Wills Point Population and Flow Projections	IX-6
Table IX-6:	City of Wills Point WWTP Facility Needs to Meet Level II Effluent Requirements and 2050 Flows	IX-7
Table IX-7:	City of Wills Point WWTP Facility Needs to Meet Level III Effluent Requirements and 2050 Flows	IX-8





Tarrant Regional Water District Watershed Project

Cedar Creek Reservoir Wastewater Treatment Facilities Report

List of Figures

Figure ES-1:	Cedar Creek Discharge Assessment Wastewater Treatment Plant Location	ES-2
Figure I-1:	Athens Total Nitrogen Monthly Average Loads	I-3
Figure I-2:	Athens Total Phosphorous Monthly Average Loads.....	I-3
Figure I-3:	City of Athens, Texas North Wastewater Treatment Plant, Plant Schematic	I-5
Figure II-1:	Cherokee Shores Total Nitrogen Monthly Average Loads	II-3
Figure II-2:	Cherokee Shores Total Phosphorous Monthly Average Loads.....	II-3
Figure II-3:	Tecon Water Company, L.P. Cherokee Shores Wastewater Treatment Plant, Plant Schematic	II-5
Figure III-1:	East Cedar Creek Total Nitrogen Loads.....	III-3
Figure III-2:	East Cedar Creek Total Phosphorous Loads	III-3
Figure III-3:	East Cedar Creek Fresh Water Supply District North Wastewater Treatment Plant, Plant Schematic	III-5
Figure IV-1:	Eustace Total Nitrogen Monthly Average Loads.....	IV-3
Figure IV-2:	Eustace Total Nitrogen Monthly Average Loads.....	IV-3
Figure IV-3:	City of Eustace, Texas Wastewater Treatment Plant, Plant Schematic.....	IV-5
Figure V-1:	Kaufman Total Nitrogen Monthly Average Loads	V-3
Figure V-2:	Kaufman Total Phosphorous Monthly Average Loads	V-3
Figure V-3:	City of Kaufman, Texas Wastewater Treatment Plant, Plant Schematic	V-5
Figure VI-1:	Kemp Total Nitrogen Monthly Average Loads.....	VI-3
Figure VI-2:	Kemp Total Phosphorous Monthly Average Loads	VI-3
Figure VI-3:	City of Kemp, Texas Wastewater Treatment Plant, Plant Schematic	VI-5
Figure VII-1:	Mabank Total Nitrogen Monthly Average Loads	VII-3
Figure VII-2:	Mabank Total Phosphorous Monthly Average Loads.....	VII-3
Figure VII-3:	City of Mabank, Texas Wastewater Treatment Plant, Plant Schematic	VII-5
Figure VIII-1:	Terrell Total Nitrogen Monthly Average Loads	VIII-3
Figure VIII-2:	Terrell Total Phosphorous Monthly Average Loads	VIII-3
Figure VIII-3:	City of Terrell, Texas King's Creek Wastewater Treatment Plant, Plant Schematic	VIII-5
Figure IX-1:	Wills Point Total Nitrogen Monthly Average Loads	IX-3
Figure IX-2:	Wills Point Total Phosphorous Monthly Average Loads.....	IX-3
Figure IX-3:	City of Wills Point, Texas Wastewater Treatment Plant, Plant Schematic	IX-5

List of Abbreviations

BOD	Biochemical Oxygen Demand/Biological Oxygen Demand
CBOD	Carbonaceous Biological Oxygen Demand
DMR	Discharge Monitoring Reports
ENR	Engineering News Record
IP	Inorganic Phosphorous
MG	Millions of Gallons
MGD	Millions of Gallons per Day
NH3	Ammonia
NH3-N	Ammonia Nitrogen
NOx	Nitrogen Oxides
ON	Organic Nitrogen
OP	Organic Phosphorous
OPO₄	Ortho-Phosphate
OrgN	Organic Nitrogen
OrgP	Organic Phosphorous
SWD	Side Wall Depth
TCEQ	Texas Commission on Environmental Quality
TN	Total Nitrogen
TP	Total Phosphorous
TPDES	Texas Pollution Discharge Elimination System
TSS	Total Suspended Solids
UV	Ultra Violet
WWTP	Wastewater Treatment Plant

EXECUTIVE SUMMARY

EXECUTIVE SUMMARY

The Tarrant Regional Water District (District) provides raw water to numerous water suppliers in the North Texas area. The raw water originates in several of the District's reservoirs including Cedar Creek, Richland Chambers, and Eagle Mountain. As the demands for raw water increase, the need to protect the water quality becomes a critical issue. This report focuses on evaluating the various point sources of pollutants, particularly nutrients, into the Cedar Creek Reservoir watersheds. The objectives of the following report are to identify significant sources of pollutant sources, to quantify both current and long-term impacts, to evaluate the best management practices available for maintaining and improving water quality, and finally to address the costs of implementing those practices.

This report addresses the point sources, specifically wastewater treatment facilities, discharging directly into the Cedar Creek Reservoir through watershed streams that eventually enter the reservoir. Plants evaluated include:

- City of Athens' North Wastewater Treatment Plant
- Cherokee Shores Wastewater Treatment Facility
- East Cedar Creek Fresh Water Supply District North Wastewater Treatment Plant
- City of Eustace Wastewater Treatment Plant
- City of Kaufman Wastewater Treatment Plant
- City of Kemp Wastewater Treatment Plant
- City of Mabank Wastewater Treatment Plant
- City of Terrell King's Creek Wastewater Treatment Plant
- City of Will's Point Wastewater Treatment Plant

Figure ES-1 shows the locations of these plants within the Cedar Creek watershed. The evaluation of each facility included assessing the ability of each plant to properly treat projected 2050 flows under three varying discharge limit criteria. The existing permit conditions are referred to as Level I. The next level of treatment, referred to as Level II, includes reduction of phosphorus to 1.0 mg/L and total nitrogen to 10 mg/L. The most stringent level of treatment considered, referred to as Level III, includes reducing the phosphorus limit to 0.5 mg/L and total nitrogen to 5 mg/L. An assessment of facility needs is performed at all three effluent limitations for flows projected through 2050.

Sources of information for the evaluations included: site visits, interviews with the plant personnel, reviews of the existing plans and historical reports, data collected by plant personnel for the District, data acquired through Texas Commission on Environmental Quality (TCEQ), and responses to a questionnaire developed specifically for this project to aid in acquiring data and information on each plant. Each memorandum is organized as follows:

- Description of the existing TPDES permit conditions and projected future limits
- Historical effluent data
- Brief description of the existing treatment process
- Population growth and flow projections
- Facility needs and costs to meet Level I effluent limits through 2050
- Facility needs and costs to meet Level II effluent limits through 2050
- Facility needs and costs to meet Level III effluent limits through 2050

A brief summary of the assessment for each plant is provided in Table ES-1. Detailed evaluations of each plant are provided in the technical memoranda following this summary. Table ES-2 summarizes the costs associated with each of the expansions or upgrades and Table ES-3 examines the nitrogen and phosphorus loads that would be associated with the projected 2050 flows at each permit level. The average total nitrogen and total phosphorous concentrations that were determined as part of special nutrient testing were used to calculate the Level I loads at 2050 flows; this will best compare the impact of each permit level across time.



Table ES-1
Tarrant Regional Water District
Cedar Creek Lake
Wastewater Plant Dischargers Assessment

Treatment Plant & Permit Limits	Flow Projections (MGD) ¹	Existing Process Description	Facility Needs for 2050 and Estimates of Probable Construction and O&M Costs		
			Level I Existing Permit Limits	Level II ² TN = 10 mg/L TP = 1 mg/L	Level III ³ TN = 5 mg/L TP = 0.5 mg/L
ATHENS	North WWTP 2005 = 0.50 2010 = 0.54 2020 = 0.65 2030 = 0.78 2040 = 0.94 2050 = 1.14 West WWTP 2005 = 0.74 2010 = 0.82 2020 = 0.98 2030 = 1.18 2040 = 1.42 2050 = 1.70 Flow = 1.027 MGD	Bar screen, grit removal, Imhoff tanks, supernatant to tricking filters, aeration for six hours, final clarification, and chlorination. Sludge disposed on drying beds and periodically disposed of off-site.	Expand influent pumping system, replace tricking filter rock media with plastic media, duplicate aeration basin and aeration, and double drying bed size. Project cost: \$3,428,914	Add denitrifying filter to meet TN limit and alum addition for P removal. Increase in cost from Levels I and II: \$76,507	Add carbon source for denitrification, and increase alum dosage for lower P limit. Equipment is already in place. Increase in cost from Levels I and II: \$76,507
CHEROKEE SHORES	BOD ₅ = 10 mg/L (Mar. - Nov.) CBOD = 20 mg/L (Dec. - Feb.) TSS = 15 mg/L (Mar. - Nov.) TSS = 20 mg/L (Dec. - Feb.) NH ₃ = 2 mg/L (Mar. - Nov.) NH ₃ = 3 mg/L (Dec. - Feb.) Flow = 1.027 MGD	Equalization, activated sludge, final clarification, sand filtration, and chlorination. Sludge aerobically digested and disposed off-site.	Additional Annual O&M cost: \$63,000	Increase in Annual O&M cost over Level I: \$22,890	Increase in Annual O&M cost over Levels I and II: \$38,535
	2005 = 0.08 2010 = 0.09 2020 = 0.11 2030 = 0.13 2040 = 0.15 2050 = 0.18 BOD ₅ = 10 mg/L TSS = 15 mg/L NH ₃ = 5 mg/L TN = 20 mg/L TP = 2 mg/L Flow = 0.15 MGD	Existing facility is capable of treating the projected flows through the year 2050. Project cost: \$0 Additional Annual O&M cost: \$0	Add alum addition for P removal, a denitrifying filter for N reduction, and an in line mixer. Increase in cost over Level I: \$592,351	Add carbon source for denitrification, and increase alum dosage for lower P limit. Increase in cost over Levels I and II: \$64,915	Increase in Annual O&M cost over Levels I and II: \$34,230 Increase in Annual O&M cost over Level I: \$13,020

Table ES-1
Tarrant Regional Water District
Cedar Creek Lake
Wastewater Plant Dischargers Assessment

Treatment Plant & Permit Limits	Flow Projections ¹ (MGD)	Existing Process Description	Facility Needs for 2050 and Probable Construction and O&M Costs		
			Level I Existing Permit Limits	Level II ² TN = 10 mg/L TP = 1 mg/L	Level III ³ TN = 5 mg/L TP = 0.5 mg/L
EAST CEDAR CREEK FWSD BOD ₅ = 10 mg/L TSS = 15 mg/L Flow = 0.626 MGD	2005 = 0.71 2010 = 0.82 2020 = 1.00 2030 = 1.17 2040 = 1.36 2050 = 1.60	Treatment facility currently under expansion. Bar screen, two oxidation ditches w/ RAS, final clarification, sand filtration, and chlorination. Sludge is dried and landfilled.	Once treatment units are optimized, expansion required between 2030 and 2040. Add oxidation ditch, clarifier, filter, and chlorine contact basin.	Operate oxidation ditch for denitrification. Alum addition for P removal.	Denitrifying filter to meet TN limit. Add carbon source for denitrification and additional alum for lower P limit.

Table ES-1
Tarrant Regional Water District
Cedar Creek Lake
Wastewater Plant Dischargers Assessment

Treatment Plant & Permit Limits	Flow Projections (MGD) ¹	Existing Process Description	Facility Needs for 2050 and Probable Construction and O&M Costs		
			Level I Existing Permit Limits	Level II ² TN = 10 mg/L TP = 1 mg/L	Level III ³ TN = 5 mg/L TP = 0.5 mg/L
EUSTACE BOD ₅ = 30 mg/L TSS = 90 mg/L Flow = 0.125 MGD	2005 = 0.084 2010 = 0.088 2020 = 0.097 2030 = 0.106 2040 = 0.115 2050 = 0.126	Bar screen, oxidation ditch w/o RAS as pre-aeration basins, two stabilization ponds w/ 21-day detention time. Disinfection not required.	Add two 15 hp aerators, clarifier, RAS/WAS pumps and piping, and disinfection system	Denitrifying filter to meet TN limit, and additional carbon source for denitrification. Additional alum for lower P limit.	Increase in cost from Level I: \$51,005
			Project cost: \$217,930 Additional Annual O&M cost: \$63,000	Increase in Annual O&M cost over Level I: \$4,683	Increase in cost from Levels I and II: \$549,461 Increase in Annual O&M cost over Levels I and II: \$30,030

Table ES-1
Tarrant Regional Water District
Cedar Creek Lake
Wastewater Plant Dischargers Assessment

Treatment Plant & Permit Limits	Flow Projections ¹ (MGD) CBOD = 10 mg/L TSS = 15 mg/L NH ₃ = 3 mg/L (May - Oct.) NH ₃ = 5 mg/L (Nov - Apr.) Flow = 1.2 MGD	Existing Process Description	Facility Needs for 2050 and Probable Construction and O&M Costs		
			Level I Existing Permit Limits	Level II ² TN = 10 mg/L TP = 1 mg/L	Level III ³ TN = 5 mg/L TP = 0.5 mg/L
KAUFMAN	2005 = 0.73 2010 = 0.83 2020 = 1.09 2030 = 1.30 2040 = 1.47 2050 = 1.65	Bar screen, grit removal, storm water holding basins, two aeration basins w/ RAS, two final clarifiers, sand filtration and ultraviolet disinfection. Sludge belt press and lime blending. Sludge disposed offsite	Convert existing aeration system to fine bubble diffusers instead of adding aeration basins. Expand WAS and RAS system. Add IFAS to aeration basins, add final clarifier, and additional ultraviolet disinfection equipment.	Denitrifying filter to meet TN limit. Alum addition required for P removal.	Add carbon source for denitrification, and additional alum for lower P limit.
KEMP	2005 = 0.113 2010 = 0.113 2020 = 0.113 2030 = 0.113 2040 = 0.113 2050 = 0.113	Bar screen, grit removal, oxidation ditch w/ RAS, two final clarifiers, and chlorination. Sludge drying beds.	Existing facility is capable of treating the projected flows through the year 2050.	Operate oxidation ditch for denitrification. Alum addition for P removal. May require additional drying beds for alum sludge.	Increase in cost from Levels I and II: \$92,736

Table ES-1
Tarrant Regional Water District
Cedar Creek Lake
Wastewater Plant Dischargers Assessment

Treatment Plant & Permit Limits	Flow Projections (MGD) ¹	Existing Process Description	Facility Needs for 2050 and Probable Construction and O&M Costs		
			Level I Existing Permit Limits	Level II ² TN = 10 mg/L TP = 1 mg/L	Level III ³ TN = 5 mg/L TP = 0.5 mg/L
MABANK CBOD = 30 mg/L TSS = 90 mg/L NH ₃ = 2 mg/L Flow = 0.4 MGD	2005 = 0.24 2010 = 0.27 2020 = 0.33 2030 = 0.38 2040 = 0.44 2050 = 0.52	Bar screen, grit removal, oxidation ditch w/o RAS followed by three stabilization ponds, and a nitrifying bio-tower. Disinfection not required. Sludge is periodically dredged from the ponds and disposed offsite.	Expansion required between 2030 and 2040. Expand headworks and pumping capacity. Add aerators to stabilization ponds and upgrade the bio-tower. Add chlorine disinfection and drying beds.	Maintain pond volume for N removal and add alum for P removal.	Add denitrifying filter to meet TN limit, carbon source for denitrification and additional alum for lower P limit.
			Project cost: \$635,242 Additional Annual O&M cost: \$98,700	Increase in cost from Level I: \$127,512 Increase in Annual O&M cost over Level I: \$5,355	Increase in cost from Levels I and II: \$835,783 Increase in Annual O&M cost over Levels I and II: \$31,290
TERRELL CBOD = 10 mg/L TSS = 15 mg/L NH ₃ = 6.0 mg/L Flow = 4.5 MGD	2005 = 3.23 2010 = 3.42 2020 = 4.19 2030 = 4.87 2040 = 5.32 2050 = 5.76 ³	Flow equalization, bar screen, grit removal, primary clarification, two-stage trickling filter, final clarification, with chlorination and dechlorination. Portable belt press solids dewatering.	Expansion required between 2020 and 2030. Add additional headworks with screens and grit removal. Add influent pump capacity, Sequencing Batch Reactors and sand filters. Expand chlorine contact basin and add gravity belt thickener.	Add denitrifying filters to existing treatment train. Optimize SBRs for denitrification. Alum addition for P removal.	Carbon source for denitrification and additional alum for lower P limit.
			Project cost: \$7,416,562 Additional Annual O&M cost: \$101,115	Increase in cost from Level I: \$3,192,437 Increase in Annual O&M cost over Level I: \$15,855	Increase in cost from Levels I and II: \$146,059 Increase in Annual O&M cost over Levels I and II: \$178,500

Table ES-1
Tarrant Regional Water District
Cedar Creek Lake
Wastewater Plant Dischargers Assessment

Treatment Plant & Permit Limits	Flow Projections ¹ (MGD) <u>Present</u> BOD ₅ = 30 mg/L TSS = 90 mg/L Flow = 0.51 MGD <u>Planned</u> CBOD = 10 mg/L TSS = 15 mg/L NH ₃ = 3 mg/L Flow = 0.80 MGD	Existing Process Description	Facility Needs for 2050 and Probable Construction and O&M Costs		
			Level I Existing Permit Limits	Level II ² TN = 10 mg/L TP = 1 mg/L	Level III ³ TN = 5 mg/L TP = 0.5 mg/L
WILLS POINT		<p>Two new stabilization ponds are currently under expansion. Bar screen, two facultative lagoons, three stabilization ponds exist. Disinfection not required. Sludge is periodically dredged from the ponds and disposed offsite.</p> <p>Once current expansion is complete, facility is capable of treating the projected flows through the year 2050.</p>	<p>Operate ponds for denitrification, and dredge ponds to maintain volume. Add earthen pond divider. Alum addition for P removal.</p> <p>Project cost: \$0</p> <p>Additional Annual O&M cost: \$0</p>	<p>Increase in cost from Levels I and II: \$835,783</p> <p>Increase in Annual O&M cost over Level I: \$4,935</p>	<p>Denitrifying filter to meet TN limit., with possible additional carbon source. Additional alum for lower P limit.</p>

¹ Flow projections based on TWDB population projections and 100 gpcd unless otherwise noted.

² Level II O&M costs are in addition to current operating costs.

³ Level III capital costs are in addition to Level I costs. Level III capital costs are in addition to Level I and Level II costs.

The initial opinion of costs presented herein represents a Class 5 estimate based upon ANSI Standard Reference Z94.2-1989 and AACE International Recommended Practice No. 17R-97.

Table ES-2: Comparison of Costs for Each Plant's Expansion or Upgrade

City or Facility	Costs Associated with Upgrades		
	Level I	Level II	Level III
Athens	\$3,428,914	\$893,743	\$76,507
Cherokee Shores	\$0	\$592,351	\$64,915
East Cedar Creek	\$4,915,008	\$127,512	\$912,290
Eustace	\$217,930	\$51,005	\$549,461
Kaufman	\$2,088,878	\$1,659,974	\$92,736
Kemp	\$0	\$51,005	\$537,869
Mabank	\$635,242	\$127,512	\$835,783
Terrell	\$7,416,562	\$3,192,437	\$146,059
Wills Point	\$0	\$231,840	\$835,783
Total Costs	\$18,702,534	\$6,927,379	\$4,051,403

Table ES-3: Nutrient Loads in Lbs/day and Tons/year for Each Plant at Each Permit Level

City or Facility	Loads Associated with 2050 Flows (lbs/day)						Loads Associated with 2050 Flows (tons/yr)					
	Total Nitrogen			Total Phosphorous			Total Nitrogen			Total Phosphorous		
	Level I	Level II	Level III	Level I	Level II	Level III	Level I	Level II	Level III	Level I	Level II	Level III
Athens	129	95	48	27	10	5	23	17	9	5	2	1
Cherokee Shores	57	15	8	6	2	0.8	10	3	1	1	0.27	0.14
East Cedar Creek	312	133	67	29	13	7	57	24	12	5	2	1
Eustace	25	11	5	5	1	0.5	5	2	1	1	0.19	0.10
Kaufman	186	138	69	39	14	7	34	25	13	7	3	1
Kemp	14	9	5	3	0.9	0.5	2	2	1	1	0.17	0.09
Mabank	51	43	22	17	4	2	9	8	4	3	1	0.40
Terrell	947	480	240	194	48	24	173	88	44	35	9	4
Wills Point	51	43	21	11	4	2	9	8	4	2	1	0.39
Level II Loads	1,771	967	484	332	97	48	323	177	88	61	18	9
Load Reduction from Level I to Level II	804			235			147			43		
Load Reduction from Level II to Level III				48						88		9

* Level I loads calculated from average TN and TP as reported in each chapter's Table 3

SECTION I

CITY OF ATHENS NORTH WASTEWATER TREATMENT PLANT

SECTION I

CITY OF ATHENS NORTH WASTEWATER TREATMENT PLANT

INTRODUCTION

The Athens North WWTP discharges into Caney Creek and then flows into the Cedar Creek Reservoir in Segment 0818 of the Trinity River Basin. Sources of information for the evaluation included: a site visit, an interview with the plant personnel, review of the existing plans and historical reports, data collected by plant personnel for the District, and the data acquired through Texas Commission on Environmental Quality (TCEQ). This section is organized as follows:

- Description of the existing TPDES permit conditions and projected future limits
- Historical effluent data
- Brief description of the treatment process
- Population growth and flow projections
- Facility needs and costs to meet Level I effluent limits through 2050
- Facility needs and costs to meet Level II effluent limits through 2050
- Facility needs and costs to meet Level III effluent limits through 2050

EXISTING PERMIT CONDITIONS

The City of Athens operates two wastewater treatment plants. One is referred to as the North plant and the other as the West. The West plant, which does not discharge into the Cedar Creek watershed, is permitted for a flow of 1.32 million gallons per day (MGD). The West plant operates under TPDES Permit No. 10143-001. Since the West plant does not discharge into the Cedar Creek sewershed, an evaluation of the WWTP is not presented in this study.

The North plant discharges into the Cedar Creek sewershed and has a rated capacity of 1.027 MGD. The plant presently treats about 40% of the City's wastewater. The North wastewater treatment plant operates under TPDES Permit No. 10143-001-A with the current permit limits for the North plant as shown in Table I-1; limits shown are the current permit limits, which may have changed throughout the course of historical data collection and report writing.

Table I-1
City of Athens North WWTP
Current TPDES Permit Conditions
TPDES Permit 10143-001-A

Parameters	Permit Limits March - November	Permit Limits December - February
Carbonaceous Biochemical Oxygen Demand (CBOD)	10 mg/L	20 mg/L
Total Suspended Solids (TSS)	15 mg/L	20 mg/L
Ammonia Nitrogen (NH ₃)	2 mg/L	3 mg/L
Annual Average Flow	1.027 MGD	1.027 MGD

The existing permit conditions are referred to as Level I. The next level of treatment, referred to as Level II, includes reduction of phosphorus to 1 mg/L and total nitrogen to 10 mg/L. The most stringent level of treatment considered, referred to as Level III, includes reduction of phosphorus to 0.5 mg/L and

total nitrogen to 5 mg/L. The assessment of facility needs is performed at all three levels for flows projected for 2050.

HISTORICAL PLANT FLOWS AND EFFLUENT QUALITY

A summary of the historical flows and effluent quality for the City of Athens' North plant are presented in Table I-2; all data in this table were derived from the plant Discharge Monitoring Reports (DMRs). The flow to the plant is currently about 40% of its rated capacity.

Table I-2
City of Athens North WWTP
Historical Plant Flows and Effluent Quality

Year	Average Daily Flow	Effluent		
		CBOD	TSS	NH ₃ -N
		(MGD)	(mg/L)	(mg/L)
1999	0.42	9.1	7.9	3.7
2000	0.50	10.5	8.3	4.4
2001	0.45	7.8	9.8	3.1
2002	0.45	3.4	7.5	1.9
2003	0.42	3.1	8.7	0.6

The North plant typically meets its permit limits for CBOD and TSS. It has met its ammonia limit for the most recent years of data collection. At the request of the Tarrant Regional Water District, the city of Athens collected 12 months of effluent nutrient data. The data indicate that the plant typically meets its ammonia limit with occasional excursions. Operational and facility changes will be required to meet Level II and III effluent goals. A summary of the nutrient data is presented in Table I-3; all data may be found in Appendix A.

Table I-3
City of Athens WWTP
Special Nutrient Testing for Tarrant Regional Water District

	NH ₃ -N	NO _x	IP	Org P	Org N	TN	TP
No. of Data Points	49	49	49	49	49	49	49
Average (mg/L)	1.68	10.91	1.72	1.16	1.34	13.53	2.85
Minimum (mg/L)	0.20	0.19	0.84	0.00	0.00	0.55	1.40
Maximum (mg/L)	20.00	27.00	3.70	2.40	8.05	27.52	4.40

Figures I-1 and I-2 illustrate the seasonal variability of total nitrogen and phosphorous loads.

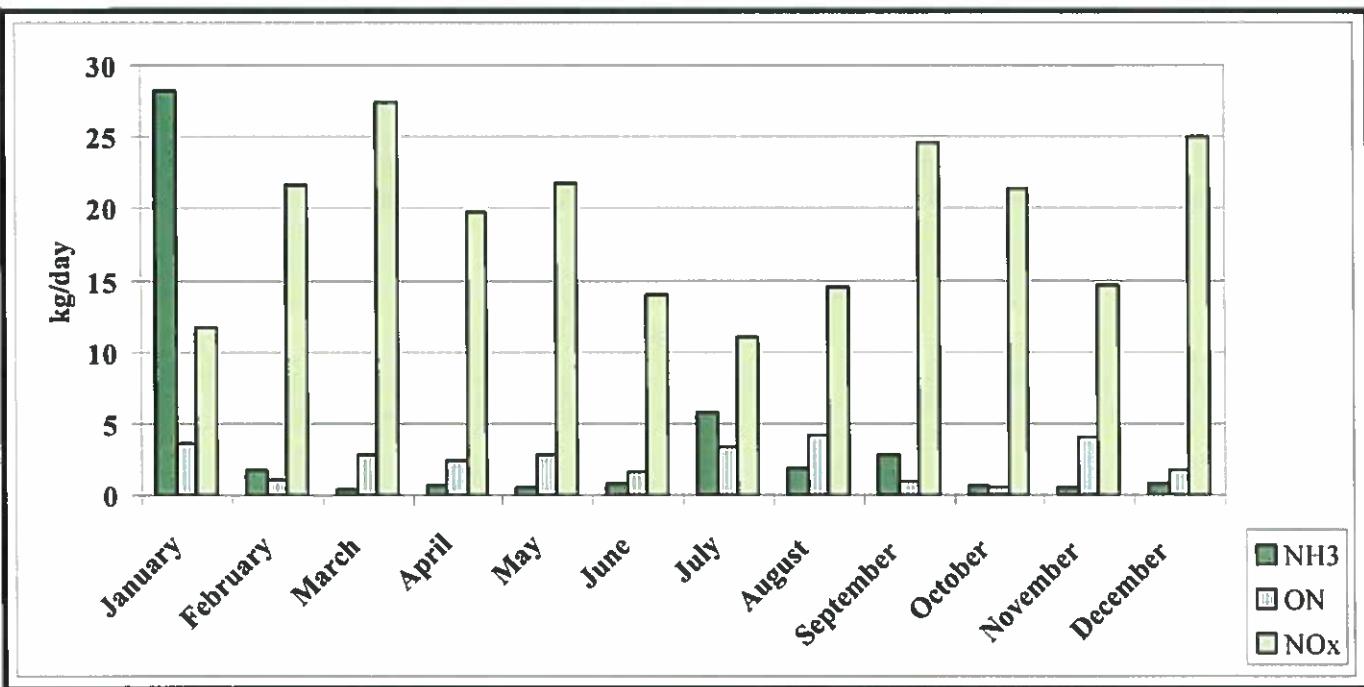


Figure I-1: Athens Effluent Total Nitrogen Monthly Average Loads

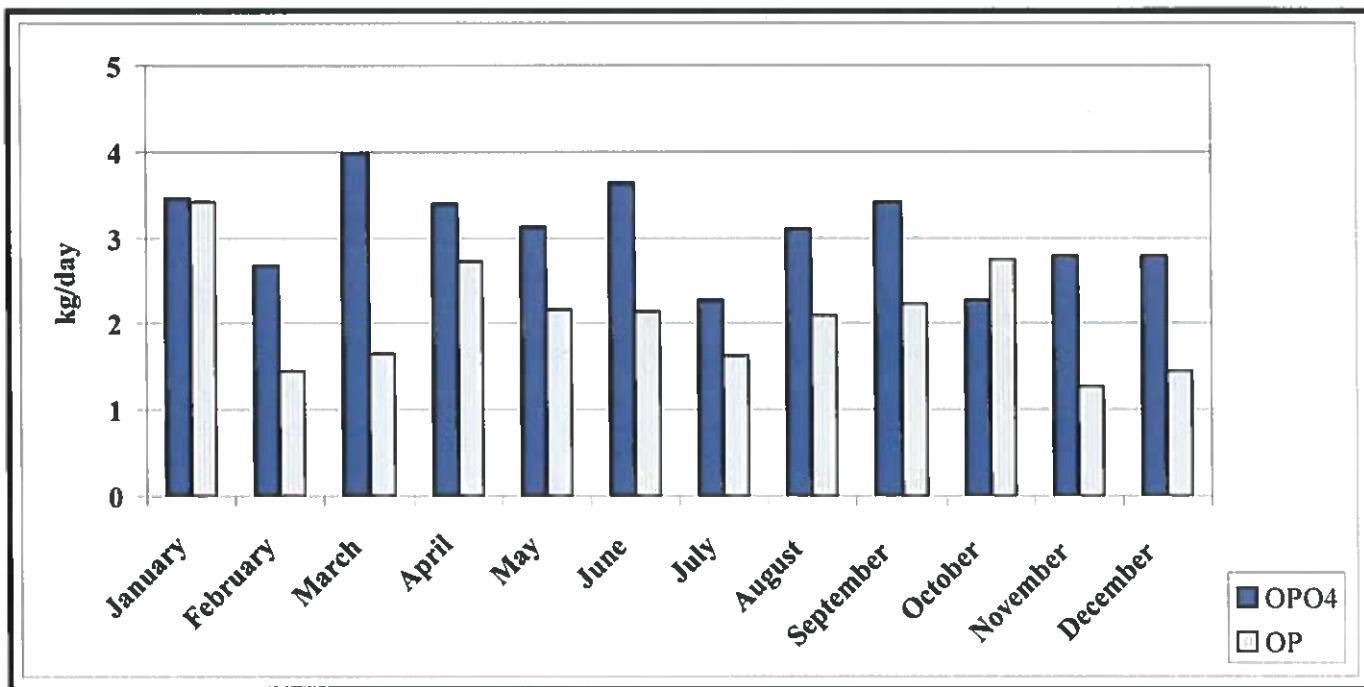


Figure I-2: Athens Effluent Total Phosphorous Monthly Average Loads

PROCESS OVERVIEW

The City's North plant is a conventional wastewater treatment plant utilizing an Imhoff tank for primary settling, trickling filters, aeration basins, and final clarification for treatment. See Figure I-3 for the plant's process schematic. Table I-4 lists the existing treatment units and their dimensions.

**Table I-4
City of Athens North WWTP
Existing Treatment Facilities**

Treatment Units	Dimensions/Size
Imhoff Tank No. 1 Volume	517,000 gallons
Imhoff Tank No. 2 Volume	96,900 gallons
Trickling Filters Nos. 1 and 2	60-ft diameter, 12-ft SWD
Aeration Basin	20-ft x 10-ft, 15-ft SWD 22,400 gallons
Final Clarifiers Nos. 1 and 2	40-ft diameter, 12-ft SWD 112,800 gallons
Chlorine Contact Basin Volume	92,000 gallons

Wastewater from the City of Athens enters the treatment facility through an 18-inch diameter gravity sewer line. The raw wastewater is passed through a manually cleaned bar screen to remove larger solids after which the grit is removed. The raw wastewater is pumped into Imhoff tanks. Settling occurs in the top compartment and degradation of the sludge under absence of oxygen occurs in the lower unheated compartment. The supernatant from the Imhoff tanks is routed to a trickling filter for further treatment of organic matter present in the wastewater; Trickling Filter No. 1 utilizes rock while No. 2 employs a plastic media. The trickling filter effluent is aerated in an aeration basin for nitrification. From the aeration basin, flow is sent to secondary clarifiers to separate the liquid and the biomass. The supernatant from the secondary clarifiers is then disinfected with chlorine solution and discharged into a creek.

Primary sludge and biological sludge are periodically wasted to the sludge drying beds. Decant water from the sludge drying bed is mixed with the raw wastewater prior to the Imhoff tanks. Dried sludge is periodically removed from the drying beds and disposed of off-site.

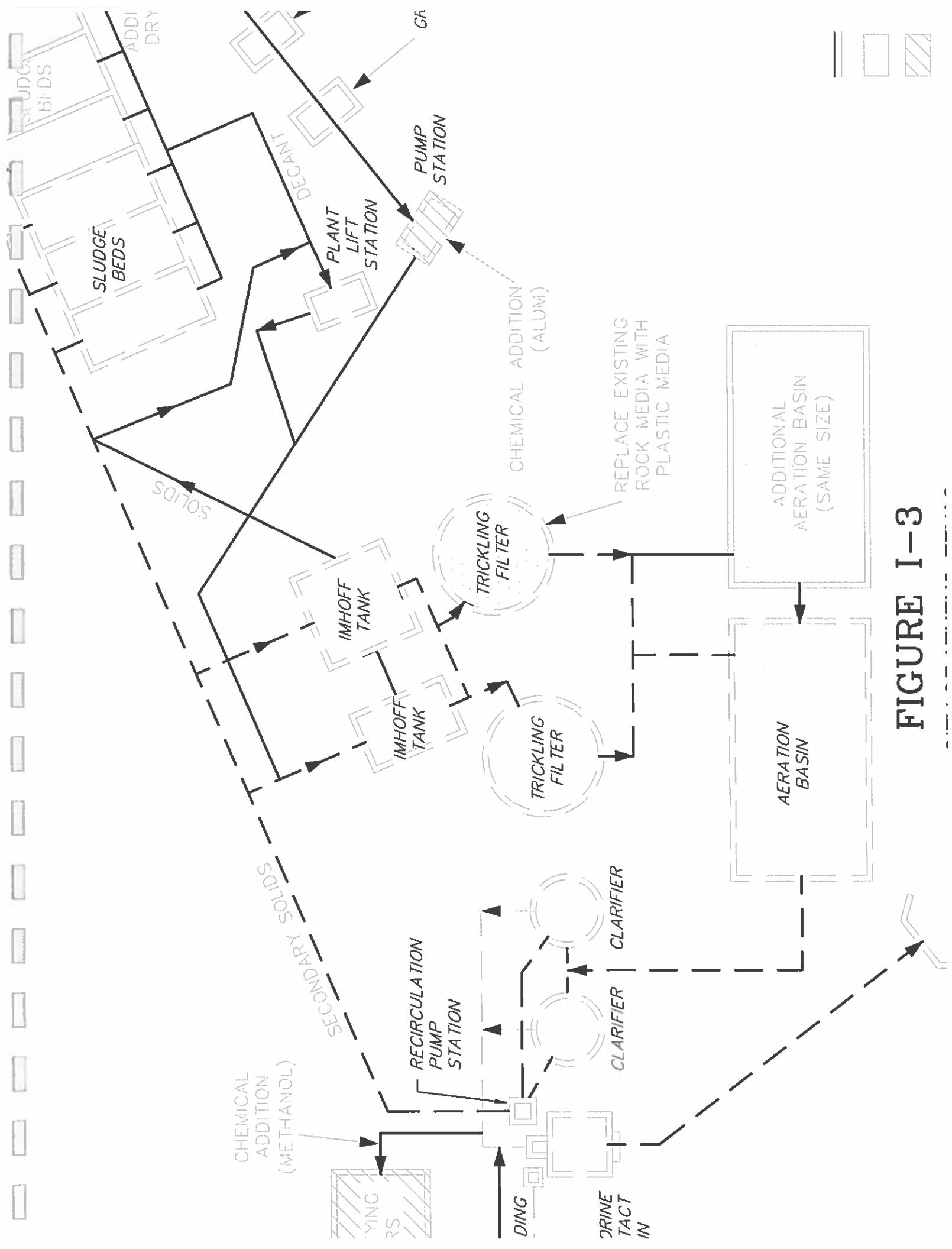


FIGURE I-3

FUTURE FLOW PROJECTIONS

In the year 2000, the population of the City of Athens was 11,297. Projected populations and flows through the year 2050 are listed in Table I-5.

**Table I-5
City of Athens
Population and Flow Projections**

Year	2005	2010	2020	2030	2040	2050
Population ¹	12,390	13,588	16,343	19,657	23,643	28,438
Projected Flow (MGD) ² North WWTP	0.50	0.54	0.65	0.78	0.94	1.14
Projected Flow (MGD) ² West WWTP	0.74	0.82	0.98	1.18	1.42	1.7

¹ Texas Water Development Board 2006 Regional Water Plan

² Average Flow = assumed 100 gal/capita/day (Figure 1, 30 TAC 317.4 (a))

Table I-5 flow projections are based on the assumption that the flow split between the North (40%) and West (60%) WWTPs will continue as it is currently. The projected 2050 flow for the North plant is 1.14 MGD. The projected flow will exceed the rated capacity of the North plant around the year 2045.

FACILITY NEEDS EVALUATION

This section describes the facilities identified for the plant to meet each of the three levels of treatment for flows through the year 2050. It should be noted that many of the existing facilities will be beyond their useful lives by the year 2050. The identification of the facility needs does not include the replacement of worn out equipment or treatment units.

Level I Facility Needs for 2050 Wastewater Flows

Level I treatment requirements are identified based on continuing to meet its existing effluent limits. As mentioned above, the plant's current permitted daily average flow is 1.027 MGD. Around the year 2045, the projected flow will exceed the current plant capacity, and there will be a need to expand the plant.

To treat year 2050 projected flows, the existing plant needs to be expanded to 1.14 MGD. An additional capacity of 0.109 MGD is required. The North WWTP needs to expand its influent pumping system to handle the higher flows. One of the existing trickling filters has rock media that can be replaced with plastic media to expand its treatment capability. An aeration basin of similar size to the existing basin needs to be added to achieve treatment capacity to treat year 2050 projected flows. Three additional sludge-drying beds of existing bed size would provide sufficient capacity through the year 2050.

The capital costs involved to improve the City's plant to treat the year 2050 flow are listed in Table I-6, as well as additional annual operation costs.

Table I-6
City of Athens North WWTP
Facility Needs to Meet Level I Effluent Requirements and 2050 Flows

Item Description		Cost
Influent Pumping		\$139,000
Replace Rock Media with Plastic Media		\$94,000
New Aeration Basin		\$1,186,000
Three Additional Drying Beds		\$60,000
	Sub-Total	\$1,479,000
Site Work and Yard Piping	20%	\$295,800
	Sub-Total	\$1,774,800
Electrical and Instrumentation	20%	\$354,960
	Sub-Total	\$2,129,760
Contingencies	25%	\$532,440
Contractor OHP	15%	\$319,464
	Total Construction Cost	\$2,981,664
Engineering	10%	\$298,166
Permitting	5%	\$149,083
	Total Project Cost	\$3,428,914
Power	75 hp and \$0.12 per kW-hr	\$60,000
Chemical		\$0
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5% \$3,000
	Total O&M Cost per Year	\$63,000

Level II Facility Needs for 2050 Wastewater Flows

The Level II effluent set facility needs are based on reducing the effluent phosphorus concentration to 1 mg/L, total nitrogen to 10 mg/L, CBOD to 10 mg/L, TSS to 15 mg/L, and ammonia nitrogen to 2 mg/L. As total phosphorus average concentrations measured during the nutrient testing were greater than 1 mg/L, this evaluation assumes phosphorus effluent concentration reduction is necessary. To consistently achieve phosphorus concentrations of less than 1 mg/L, the addition of aluminum sulfate (alum) is recommended. Alum solution will be fed at the influent pump station to achieve required mixing of the alum solution. The use of alum may retard drying of the sludge on the drying beds. It may be necessary to add more drying beds or modify the existing ones to accommodate alum sludge.

A denitrifying filter will be added to meet the effluent total nitrogen limits. An alternative to the denitrifying filter would involve constructing anoxic zones in the aeration basin and recirculating nitrified effluent to the anoxic zone. This alternative should be evaluated during preliminary design.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level II effluent requirements are listed in Table I-7.

Table I-7
City of Athens North WWTP
Facility Needs to Meet Level II Effluent Requirements and 2050 Flows

Item Description		Cost
Denitrification Filter		\$330,500
Chemical Feed System for Alum		\$55,000
	Sub-Total	\$385,500
Site Work and Yard Piping	20%	\$77,100
	Sub-Total	\$462,600
Electrical and Instrumentation	20%	\$92,520
	Sub-Total	\$555,120
Contingencies	25%	\$138,780
Contractor OHP	15%	\$83,268
	Total Construction Cost	\$777,168
Engineering	10%	\$77,717
Permitting	5%	\$38,858
	Total Project Cost	\$893,743
Power	25 hp and \$0.12 per kW-hr	\$20,000
Chemical		\$1,800
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5%
		\$1,090
	Total O&M Cost per Year	\$22,890

Level III Facility Needs for 2050 Wastewater Flows

The Level III effluent set facility needs are based on reducing the effluent phosphorus concentration to 0.5 mg/L, total nitrogen to 5 mg/L, CBOD to 10 mg/L, TSS to 15 mg/L, and ammonia nitrogen to 2 mg/L. To achieve the Level III concentrations of phosphorus, higher dosages of alum will be required. Additional carbon may be required at the denitrifying filters to reduce total nitrogen levels to less than 5 mg/L.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level III effluent requirements are listed in Table I-8.

Table I-8
City of Athens North WWTP
Facility Needs to Meet Level III Effluent Requirements and 2050 Flows

Item Description		Cost
Chemical Feed System for Methanol		\$33,000
	Sub-Total	\$33,000
Site Work and Yard Piping	20%	\$6,600
	Sub-Total	\$39,600
Electrical and Instrumentation	20%	\$7,920
	Sub-Total	\$47,520
Contingencies	25%	\$11,880
Contractor OHP	15%	\$7,128
	Total Construction Cost	\$66,528
Engineering	10%	\$6,653
Permitting	5%	\$3,326
	Total Project Cost	\$76,507
Power		\$0
Chemical		\$36,700
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5%
		\$1,835
	Total O&M Cost per Year	\$38,535

SECTION II

CHEROKEE SHORES WASTEWATER TREATMENT PLANT

SECTION II

CHEROKEE SHORES WASTEWATER TREATMENT PLANT

INTRODUCTION

The Cherokee Shores wastewater treatment facility (WWTF) discharges into the Cedar Creek Reservoir in Segment 0818 of the Trinity River Basin. Sources of information for the evaluation included: a site visit, an interview with the plant personnel, review of the existing plans and historical reports, data collected by plant personnel for the District, and the data acquired through Texas Commission on Environmental Quality (TCEQ). This section is organized as follows:

- Description of the existing TPDES permit conditions
- Historical effluent data
- Brief description of the treatment process
- Population growth and flow projections
- Facility needs and costs to meet Level I effluent limits through 2050
- Facility needs and costs to meet Level II effluent limits through 2050
- Facility needs and costs to meet Level III effluent limits through 2050

EXISTING PERMIT CONDITIONS

Tecon Water Company, L.P., owns and operates the Cherokee Shores wastewater treatment facility. The wastewater treatment plant is operated under TPDES Permit No. 13879-001 and has a rated capacity of 0.15 million gallons per day (MGD). The permit limits are shown in Table II-1; limits shown are final permit limits, which may have changed throughout the course of historical data collection and report writing.

Table II-1
Cherokee Shores WWTF
Current TPDES Permit Conditions
TPDES Permit No. 13879-001

Parameters	Permit Limits
BOD ₅	10 mg/L
Total Suspended Solids (TSS)	15 mg/L
Ammonia Nitrogen (NH ₃)	5 mg/L
Total Nitrogen	20 mg/L
Total Phosphorus	2 mg/L
Flow	0.15 MGD

The existing permit conditions are referred to as Level I. The next level of treatment, referred to as Level II, includes reduction of phosphorus to 1 mg/L and total nitrogen to 10 mg/L. The most stringent level of treatment considered, referred to as Level III, includes reduction of phosphorus to 0.5 mg/L and total nitrogen to 5 mg/L. The assessment of facility needs is performed at all three levels for flows projected for 2050.

HISTORICAL PLANT FLOWS AND EFFLUENT QUALITY

A summary of the historical flows and effluent quality for Cherokee Shores wastewater treatment facility are presented in Table II-2; all data in this table were derived from the plant Discharge Monitoring Reports (DMRs). As shown, the flow to the plant is currently about one-half of its rated capacity.

Table II-2
Cherokee Shores WWTF
Historical Plant Flows and Effluent Quality

Year	Average Daily Flow (MGD)	Effluent	
		BOD ₅ (mg/L)	TSS (mg/L)
1999	0.06	9.7	8.3
2000	0.05	11.3	11.5
2001	0.06	18.4	25.3
2002	0.08	9.4	20.4
2003	0.09	8.4	18.3
ND: No Data			

Historically, the plant has not met its effluent permit limits for TSS, and has had some difficulty in consistently meeting the BOD₅ limit. The plant has applied for and received a permit for expansion, the limits of which are those in Table II-1, and these final limits after expansion will be considered Level I for the purposes of this report. During the course of writing this report, the plant was subject to no permit limits for total nitrogen, phosphorous, or ammonia and the permitted flow was 0.075 MGD.

At the request of the Tarrant Regional Water District, Cherokee Shores collected 12 months of effluent nutrient data. During the 12 months of data collection, the plant's average nutrient concentrations were above the effluent limits for the final permit. A review of operations should be done to determine the reason for non-compliance. Operational and facility changes will be required to meet Level II and Level III effluent goals. A summary of the data is presented in Table II-3; all data may be found in Appendix A.

Table II-3
Cherokee Shores WWTF
Special Nutrient Testing for Tarrant Regional Water District

	NH ₃ -N	NO _x	IP	Org P	Org N	TN	TP
No. of Data Points	50	50	50	50	50	50	50
Average (mg/L)	30.6	1.8	3.4	0.9	5.6	37.8	4.3
Minimum (mg/L)	5.5	0.4	0.2	0.0	0.0	18.4	0.5
Maximum (mg/L)	42.0	27.0	16.5	3.4	30.3	50.7	18.6

Figures II-1 and II-2 illustrate the seasonal variability of total nitrogen and phosphorous loads.

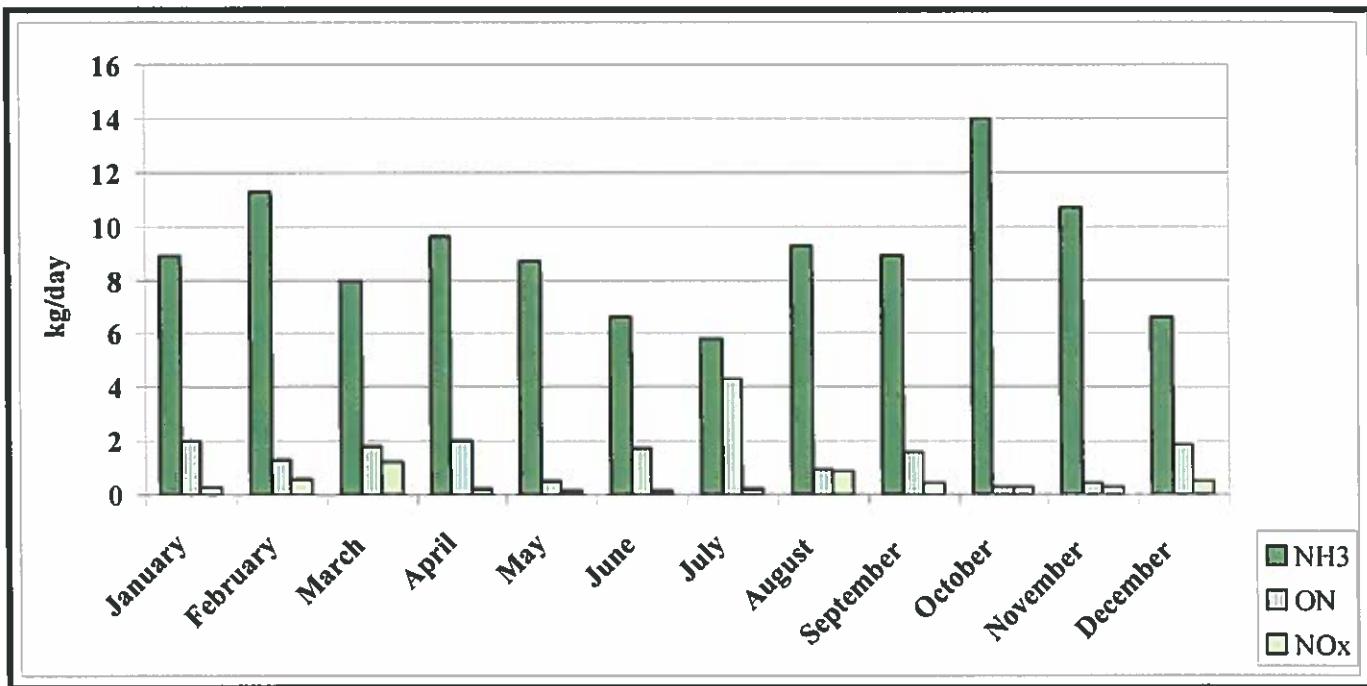


Figure II-1: Cherokee Shores Effluent Total Nitrogen Monthly Average Loads

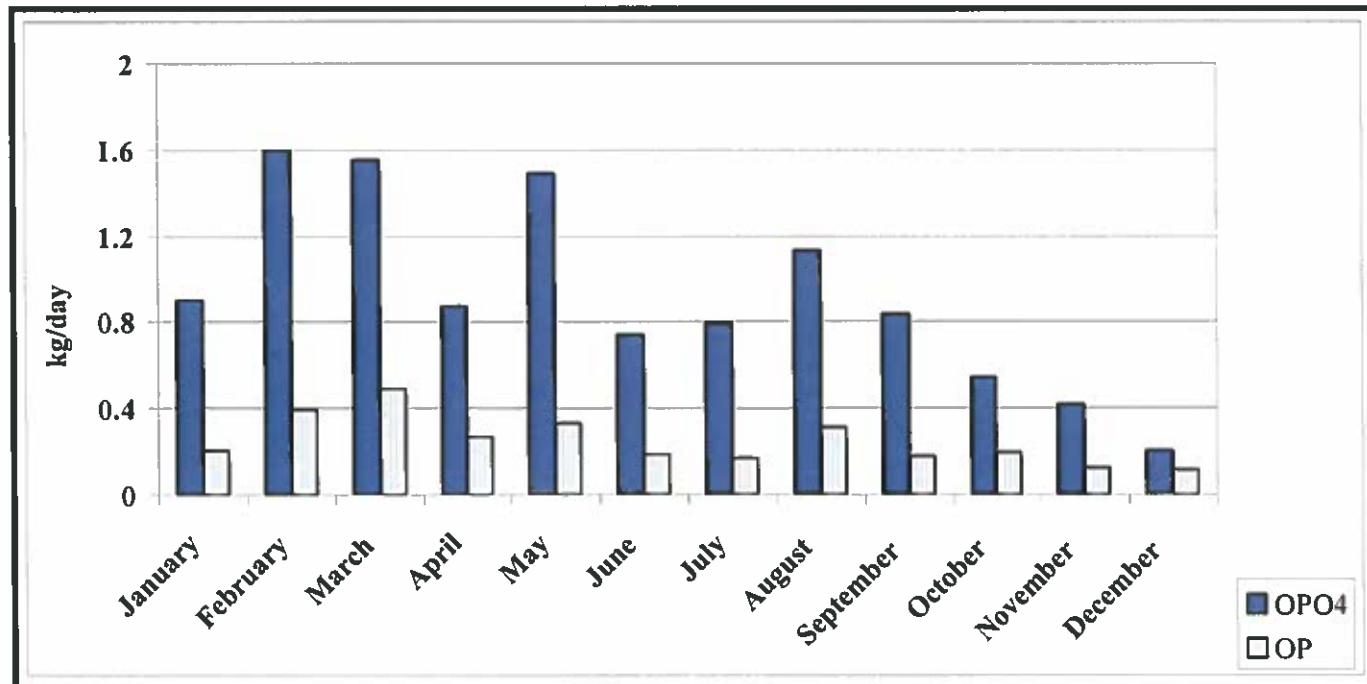


Figure II-2: Cherokee Shores Effluent Total Phosphorous Monthly Average Loads

PROCESS OVERVIEW

The Cherokee Shores WWTF is a conventional wastewater treatment plant utilizing flow equalization, aeration basins, final clarification, filtration, and disinfection for treatment. The City's WWTF rated treatment capacity will be 0.15 MGD, but at the time of this report the plant was operating under a 0.075 MGD limit. See Figure II-3 for a schematic of the facilities processes. Table II-4 lists the existing treatment units and their current sizes.

Table II-4
Cherokee Shores WWTF
Existing Treatment Facilities

Treatment Units	Dimensions/Sizes
Aeration Basin	72-ft x 12-ft, 8-ft SWD 51,700 gallons
Circular Clarifier	20-ft diameter, 6-ft SWD 14,000 gallons
Filter Surface Area	80 square feet (each)
Chlorine Contact Basin Volume	7,200 gallons
Equalization Basin Volume	2,350 gallons
Aerobic Digester Volume	16,150 gallons

Influent wastewater is either routed to the aeration basin or to a flow equalization tank. The primary purpose of the equalization tank is to hold peak flows for short time periods so as not to exceed the capacity of the other treatment units. The equalization tank can also be used to equalize flow with varying characteristics. The wastewater is treated in an aeration basin where microorganisms stabilize the organics present in the wastewater. Flow from the aeration basin is settled in the clarifier to separate out the biomass. The clarifier overflow is then passed through a sand filtration system and disinfected with chlorine solution prior to discharge.

A portion of settled sludge is returned to the aeration basin as return activated sludge and the remainder is sent to an aerobic digester. Digested sludge is pumped from the digester and disposed off-site.

Table V-7
City of Kaufman WWTP
Facility Needs to Meet Level II Effluent Requirements and 2050 Flows

Item Description		Cost
Chemical Feed System for Alum		\$55,000
Denitrification Filters		\$661,000
	Sub-Total	\$716,000
Site Work and Yard Piping	20%	\$143,200
	Sub-Total	\$859,200
Electrical and Instrumentation	20%	\$171,840
	Sub-Total	\$1,031,040
Contingencies	25%	\$257,760
Contractor OHP	15%	\$154,656
	Total Construction Cost	\$1,443,456
Engineering	10%	\$144,346
Permitting	5%	\$72,173
	Total Project Cost	\$1,659,974
Power	5 hp and \$0.12 per kW-hr	\$4,000
Chemical		\$2,600
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5% \$330
	Total O&M Cost per Year	\$6,930

Level III Facility Needs for 2050 Wastewater Flows

The Level III effluent set facility needs are based on reducing the effluent phosphorus concentration to 0.5 mg/L, total nitrogen to 5 mg/L, and ammonia nitrogen to 3 mg/L. To achieve the Level III concentrations of phosphorus, higher dosages of alum will be needed.

Minor additional capital investment is necessary for the Level II to Level III transition, as the majority the required infrastructure would be in placed in the Level II improvements. Operational cost would increase, as additional chemicals are required. Additional carbon may be required at the denitrifying filters to reduce total nitrogen levels to less than 5 mg/L.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level III effluent requirements are listed in Table V-8.

Table V-8
City of Kaufman WWTP
Facility Needs to Meet Level III Effluent Requirements and 2050 Flows

Item Description		Cost
Chemical Feed System for Methanol		\$40,000
	Sub-Total	\$40,000
Site Work and Yard Piping	20%	\$8,000
	Sub-Total	\$48,000
Electrical and Instrumentation	20%	\$9,600
	Sub-Total	\$57,600
Contingencies	25%	\$14,400
Contractor OHP	15%	\$8,640
	Total Construction Cost	\$80,640
Engineering	10%	\$8,064
Permitting	5%	\$4,032
	Total Project Cost	\$92,736
Power		
Chemical		\$52,800
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5%
	Total O&M Cost per Year	\$55,440

SECTION VI

CITY OF KEMP

WASTEWATER TREATMENT PLANT

SECTION VI

CITY OF KEMP WASTEWATER TREATMENT PLANT

INTRODUCTION

The Kemp WWTP discharges into the Cedar Creek Reservoir in Segment 0818 of the Trinity River Basin. Sources of information for the evaluation included: a site visit, an interview with the plant personnel, review of the existing plans and historical reports, data collected by plant personnel for the District, and the data acquired through Texas Commission on Environmental Quality (TCEQ). This section is organized as follows:

- Description of the existing TPDES permit conditions and projected future limits
- Historical effluent data
- Brief description of the treatment process
- Population growth and flow projections
- Facility needs and costs to meet Level I effluent limits through 2050
- Facility needs and costs to meet Level II effluent limits through 2050
- Facility needs and costs to meet Level III effluent limits through 2050

EXISTING PERMIT CONDITIONS

The City of Kemp operates its wastewater treatment plant under TPDES Permit No. 10695-001 and has a rated capacity of 0.2 million gallons per day (MGD). The permit limits are shown in Table VI-1; limits shown are existing permit limits, which may have changed throughout the course of historical data collection and report writing.

Table VI-1
City of Kemp WWTP
Current TPDES Permit Conditions
TPDES Permit No. 10695-001

Parameters	Permit Limits
Carbonaceous Biochemical Oxygen Demand (CBOD)	5 mg/L
Total Suspended Solids (TSS)	5 mg/L
Ammonia Nitrogen (NH ₃ -N)	2 mg/L
Flow	0.2 MGD

The existing permit conditions are referred to as Level I. The next level of treatment, referred to as Level II, includes reduction of phosphorus to 1 mg/L and total nitrogen to 10 mg/L. The most stringent level of treatment considered, referred to as Level III, includes reduction of phosphorus to 0.5 mg/L and total nitrogen to 5 mg/L. The assessment of facility needs is performed at all three levels for flows projected for 2050.

HISTORICAL PLANT FLOWS AND EFFLUENT QUALITY

A summary of historical flows and effluent quality for the City of Kemp's wastewater treatment plant are presented in Table VI-2; all data in this table were derived from the plant Discharge Monitoring Reports (DMRs). As shown, the flow to the plant is currently about one-half of its rated capacity.

**Table VI-2
City of Kemp WWTP
Historical Plant Flows and Effluent Quality**

Year	Average Daily Flow (MGD)	Effluent		
		CBOD (mg/L)	TSS (mg/L)	NH ₃ -N (mg/L)
1999	0.1	6.9	20.4	ND
2000	ND	ND	ND	ND
2001	ND	ND	ND	ND
2002*	0.07	2.2	8.4	0.6
2003*	0.1	2.9	11.7	0.9

ND: No Data
*Incomplete year's worth of data; average of seven and eight months, respectively

Data indicate that the plant met its CBOD and NH₃-N limit for the past two years. At the request of the Tarrant Regional Water District, the city of Kemp collected 12 months of effluent nutrient data. Operational and facility changes will be required to meet Level II and III effluent goals. A summary of the data is presented in Table VI-3; all data may be found in Appendix A.

**Table VI-3
City of Kemp WWTP
Special Nutrient Testing for Tarrant Regional Water District**

	NH ₃ -N	NO _x	IP	Org P	Org N	TN	TP
No. of Data Points	53	53	53	53	53	53	53
Average (mg/L)	0.39	11.92	2.73	0.45	2.05	14.36	3.18
Minimum (mg/L)	0.02	0.00	0.50	0.00	0.06	1.65	0.93
Maximum (mg/L)	3.00	29.70	7.46	3.32	4.82	37.52	8.55

Figures VI-1 and VI-2 illustrate the seasonal variability of total nitrogen and phosphorous loads.

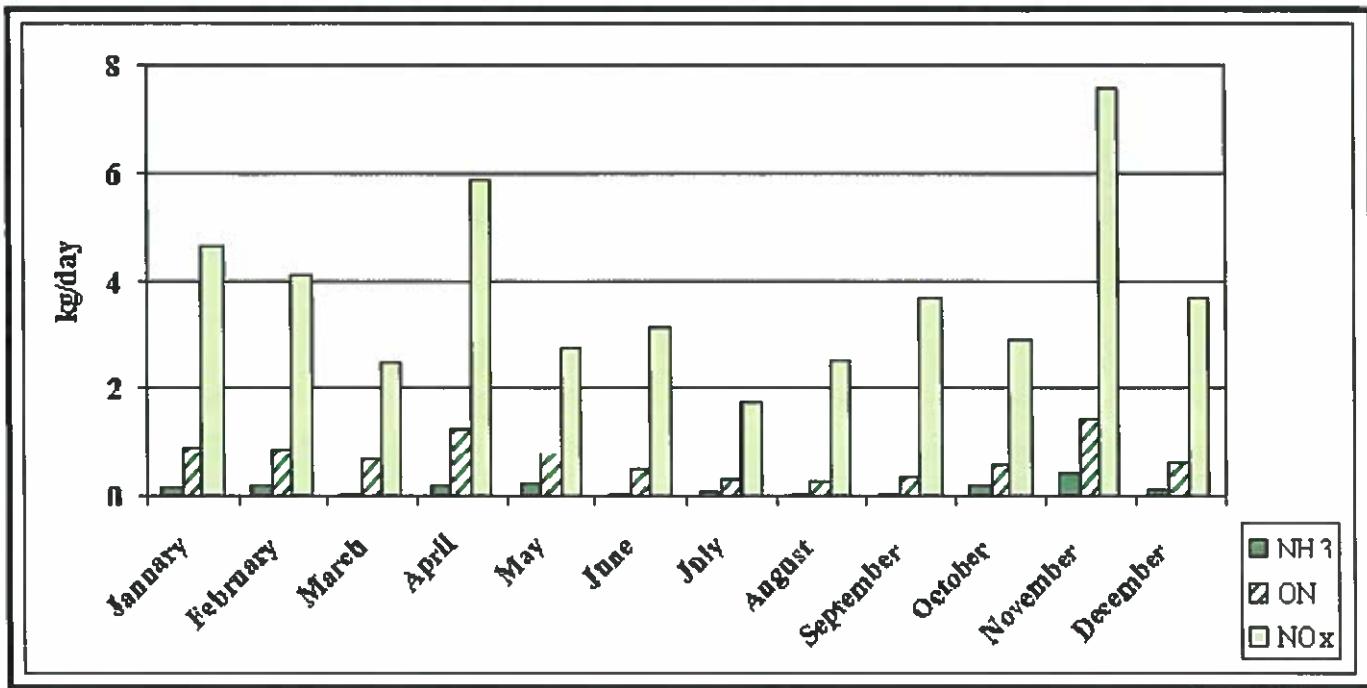


Figure VI-1: Kemp Effluent Total Nitrogen Monthly Average Loads

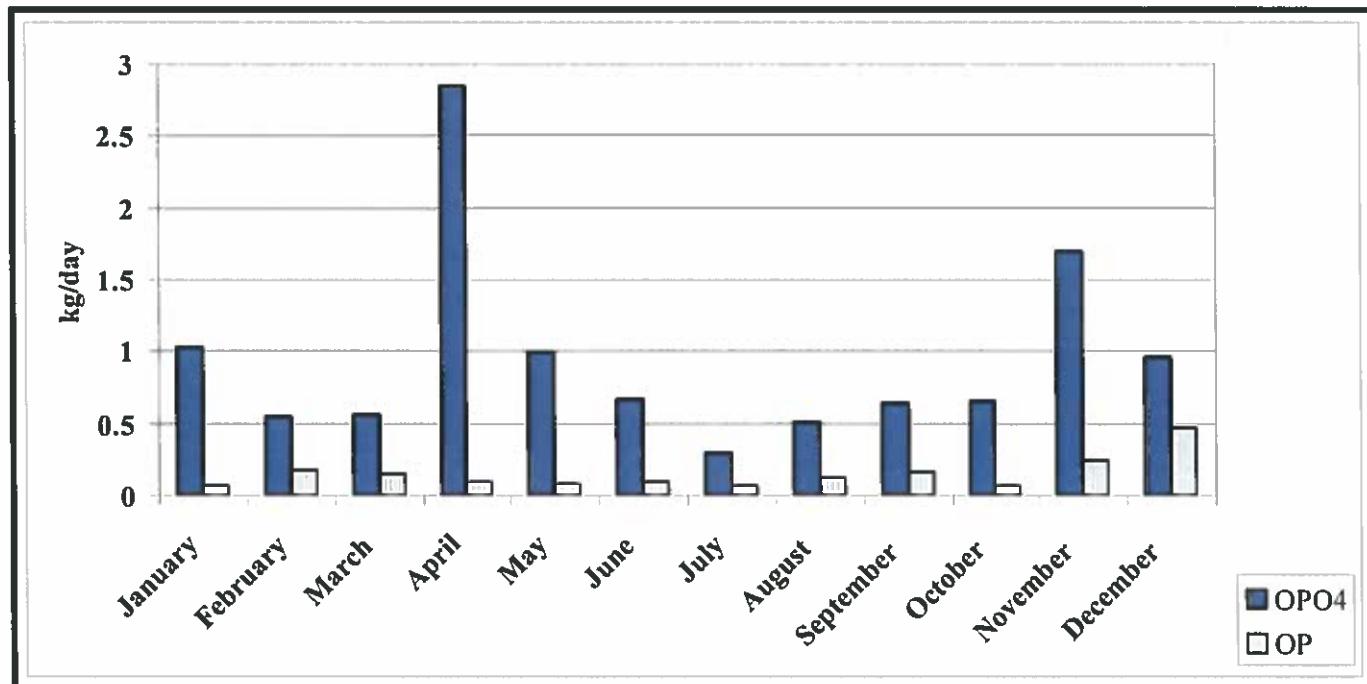


Figure VI-2: Kemp Effluent Total Phosphorous Monthly Average Loads

PROCESS OVERVIEW

The City's WWTP is a conventional wastewater treatment plant utilizing screening, grit removal, aeration basins, final clarification, and chlorine disinfection for treatment. See Figure VI-3 for the plant's process schematic. Table VI-4 lists the existing treatment units and their current sizes.

**Table VI-4
City of Kemp WWTP
Existing Treatment Facilities**

Treatment Units	Dimension/Size
Oxidation Ditch Volume	117-ft x 51-ft, 10-ft SWD 446,000 gallons
Circular Clarifier (typ.2) Volume	28-ft diameter, 10-ft SWD 46,000 gallons (each)
Chlorine Contact Basin Volume	17,200 gallons
Sludge Drying Beds Total Area	5,644 square feet

Wastewater from the collection system passes through a bar screen and continues through a grit removal system at the headworks. The wastewater then flows into an oxidation ditch, where the wastewater is mixed with return activated sludge from the clarifier. The activated sludge is then settled in the circular clarifiers. The clarified effluent is disinfected with chlorine prior to discharge. The portion of sludge settled in the clarifier is returned to aeration basin as return activated sludge and the remaining portion is dewatered in sludge drying beds.

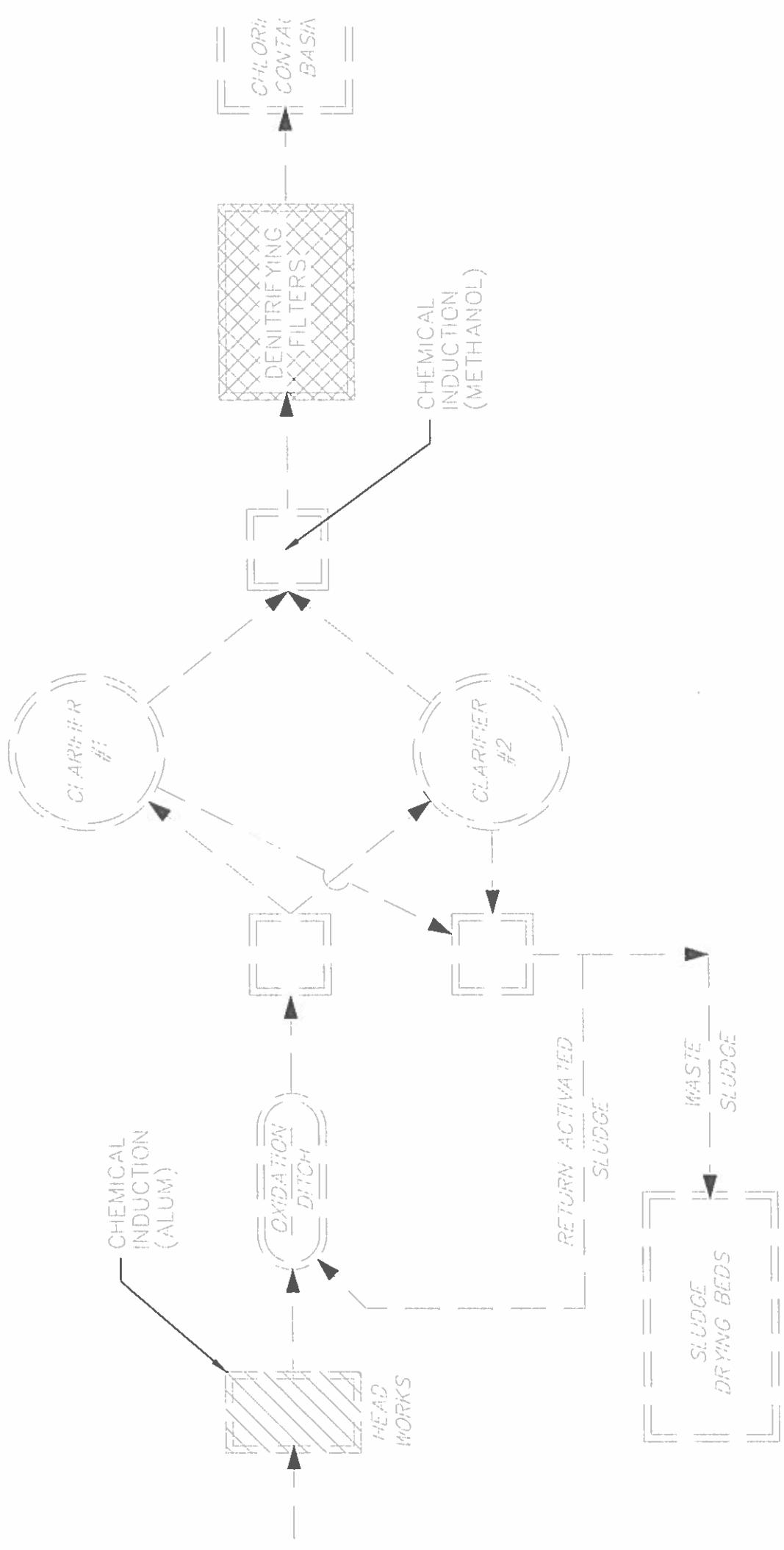


FIGURE VI-3
CITY OF KEMPTON TEXAS

FUTURE FLOW PROJECTIONS

In the year 2000, the population of the City of Kemp was 1,133. Projected populations and flows through the year 2050 are listed in Table VI-5.

Table VI-5
City of Kemp
Population and Flow Projections

Year	2005	2010	2020	2030	2040	2050
Population ¹	1,133	1,133	1,133	1,133	1,133	1,133
Projected Flow (MGD) ²	0.113	0.113	0.113	0.113	0.113	0.113

¹ Texas Water Development Board 2006 Regional Water Plan

² Average Flow = 100 gal/capita/day (Figure 1: 30 TAC 317.4 (a))

The City of Kemp plant has enough capacity to treat the projected flows through the year 2050.

FACILITY NEEDS EVALUATION

This section describes the facilities identified for the plant to meet each of the three levels of treatment for flows through the year 2050. It should be noted that many of the existing facilities will be beyond their useful lives by the year 2050. The identification of the facility needs does not include the replacement of worn out equipment or treatment units.

Level I Facility Needs for 2050 Wastewater Flows

Level I treatment requirements are identified based on continuing to meet its existing effluent limits. As mentioned above, the permitted daily average flow through the City's plant is 0.2 MGD and the current plant is capable of treating the projected flows through the year 2050.

Level II Facility Needs for 2050 Wastewater Flows

The Level II effluent set facility needs are based on reducing the effluent phosphorus concentration to 1 mg/L, total nitrogen to 10 mg/L, and ammonia nitrogen to 2 mg/L. The historical data shows that the total nitrogen effluent concentration averages about 15 mg/L and total phosphorus effluent concentration averages around 3 mg/L. To consistently achieve phosphorus concentrations of less than 1 mg/L, the addition of aluminum sulfate (alum) is recommended. The use of alum may retard drying of the sludge on the drying beds. It may be necessary to add drying beds or modify the existing ones to accommodate alum sludge. The existing oxidation ditch can be operated for denitrification to achieve total nitrogen removal to less than 10 mg/L.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level II effluent requirements are listed in Table VI-6.

Table VI-6
City of Kemp WWTP
Facility Needs to Meet Level II Effluent Requirements and 2050 Flows

Item Description		Cost
Chemical Feed System for Alum		\$22,000
	Sub-Total	\$22,000
Site Work and Yard Piping	20%	\$4,400
	Sub-Total	\$26,400
Electrical and Instrumentation	20%	\$5,280
	Sub-Total	\$31,680
Contingencies	25%	\$7,920
Contractor OHP	15%	\$4,752
	Total Construction Cost	\$44,352
Engineering	10%	\$4,435
Permitting	5%	\$2,218
	Total Project Cost	\$51,005
Power	5 hp and \$0.12 per kW-hr	\$4,000
Chemical		\$230
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5% \$212
	Total O&M Cost per Year	\$4,442

Level III Facility Needs for 2050 Wastewater Flows

The Level III effluent set facility needs are based on reducing the effluent phosphorus concentration to 0.5 mg/L, total nitrogen to 5 mg/L, and ammonia nitrogen to 2 mg/L. To achieve the Level III concentrations of phosphorus, higher dosages of alum will be needed. A denitrifying filter will also be required to meet the effluent total nitrogen limits. Additional carbon may be required at the denitrifying filters to reduce total nitrogen levels to less than 5 mg/L.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level III effluent requirements are listed in Table VI-7.

Table VI-7
City of Kemp WWTP
Facility Needs to Meet Level III Effluent Requirements and 2050 Flows

Item Description		Cost
Chemical Feed System for Methanol		\$28,000
Denitrification Filter		\$204,000
Sub-Total		\$232,000
Site Work and Yard Piping	20%	\$46,400
Sub-Total		\$278,400
Electrical and Instrumentation	20%	\$55,680
Sub-Total		\$334,080
Contingencies	25%	\$83,520
Contractor OHP	15%	\$50,112
Total Construction Cost		\$467,712
Engineering	10%	\$46,771
Permitting	5%	\$23,386
Total Project Cost		\$537,869
Power	20 hp and \$0.12 per kW-hr	\$16,000
Chemical		\$4,600
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5% \$1,030
Total O&M Cost per Year		\$21,630

SECTION VII

CITY OF MABANK

WASTEWATER TREATMENT PLANT

SECTION VII

CITY OF MABANK WASTEWATER TREATMENT PLANT

INTRODUCTION

The Mabank WWTP discharges into the Cedar Creek Reservoir in Segment 0818 of the Trinity River Basin. Sources of information for the evaluation included: a site visit, an interview with the plant personnel, review of the existing plans and historical reports, data collected by plant personnel for the District, and the data acquired through Texas Commission on Environmental Quality (TCEQ). This section is organized as follows:

- Description of the existing TPDES permit conditions and projected future limits
- Historical effluent data
- Brief description of the treatment process
- Population growth and flow projections
- Facility needs and costs to meet Level I effluent limits through 2050
- Facility needs and costs to meet Level II effluent limits through 2050
- Facility needs and costs to meet Level III effluent limits through 2050

EXISTING PERMIT CONDITIONS

The City of Mabank operates its wastewater treatment plant under TPDES Permit No. 10579-001 and has a rated capacity of 0.4 million gallons per day (MGD). The permit limits are shown in Table VII-1; limits shown are existing permit limits, which may have changed throughout the course of historical data collection and report writing.

Table VII-1
City of Mabank WWTP
Current TPDES Permit Conditions
TPDES Permit No. 10579-001

Parameters	Permit Limits
Carbonaceous Biochemical Oxygen Demand (CBOD)	30 mg/L
Total Suspended Solids (TSS)	90 mg/L
Ammonia Nitrogen (NH ₃)	2 mg/L
Flow	0.4 MGD

The existing permit conditions are referred to as Level I. The next level of treatment, referred to as Level II, includes reduction of phosphorus to 1 mg/L and total nitrogen to 10 mg/L. The most stringent level of treatment considered, referred to as Level III, includes reduction of phosphorus to 0.5 mg/L and total nitrogen to 5 mg/L. The assessment of facility needs is performed at all three levels for flows projected for 2050.

HISTORICAL PLANT FLOWS AND EFFLUENT QUALITY

A summary of historical flows and effluent quality for the City of Mabank's wastewater treatment plant are presented in Table VII-2; all data in this table were derived from the plant Discharge Monitoring Reports (DMRs). As shown, the flow to the plant is currently about one-third of its rated capacity. The plant is consistently meeting its current effluent permit conditions.

Table VII-2
City of Mabank WWTP
Historical Plant Flows and Effluent Quality

Year	Average Daily Flow (MGD)	Effluent		
		CBOD (mg/L)	TSS (mg/L)	NH ₃ -N (mg/L)
1999	0.17	8.0	35.9	1.1
2000*	0.16	6.8	24.1	0.3
2001	ND	ND	ND	ND
2002	ND	ND	ND	ND
2003	0.11	10.7	46.8	1.3

ND: No Data
 *Incomplete year's worth of data; only one month reported

At the request of the Tarrant Regional Water District, the city of Mabank collected 12 months of effluent nutrient data. Operational and facility changes will be required to meet Level II and III effluent goals. A summary of the data is presented in Table VII-3; all data may be found in Appendix A.

Table VII-3
City of Mabank WWTP
Special Nutrient Testing for Tarrant Regional Water District

	NH ₃ -N	NO _x	IP	Org P	Org N	TN	TP
No. of Data Points	52	52	52	52	52	52	52
Average (mg/L)	1.15	6.35	3.29	0.60	4.28	11.77	3.89
Minimum (mg/L)	0.16	1.56	1.20	0.00	0.00	6.23	1.78
Maximum (mg/L)	3.88	14.50	4.67	1.19	10.26	19.12	5.16

Figures VII-1 and VII-2 illustrate the seasonal variability of total nitrogen and phosphorous loads.

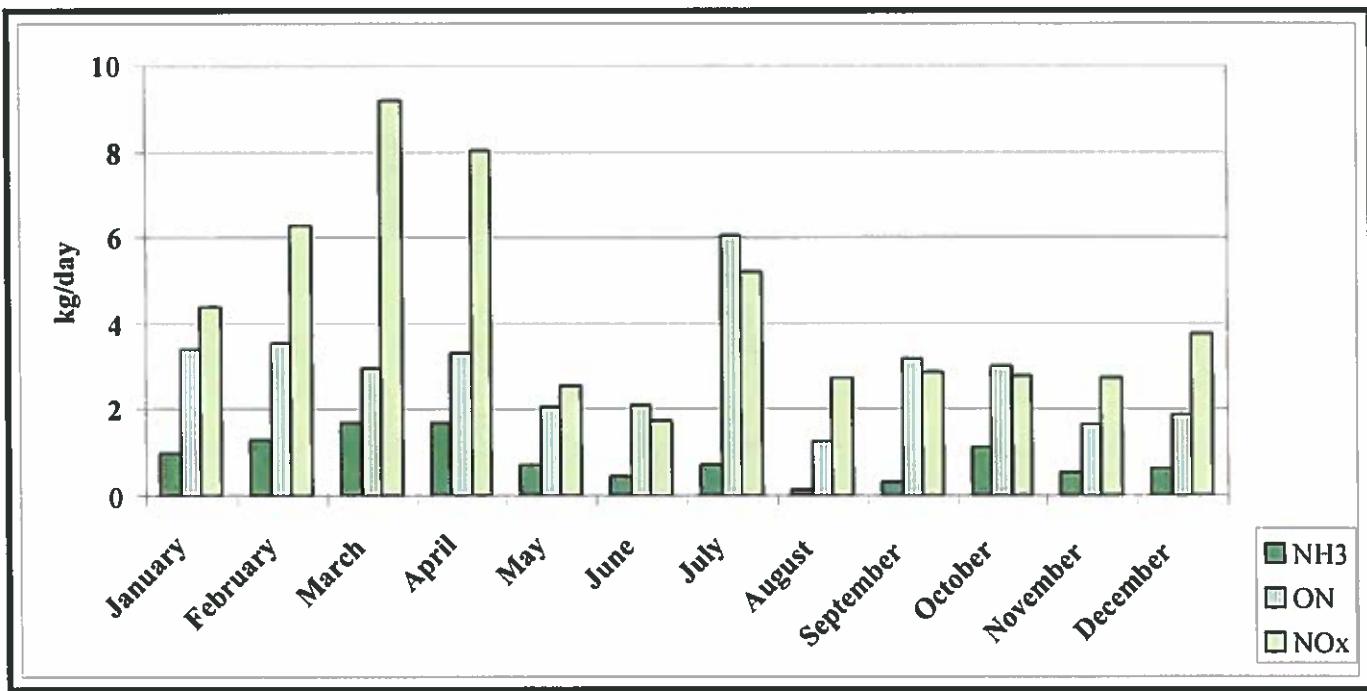


Figure VII-1: Mabank Effluent Total Nitrogen Monthly Average Loads

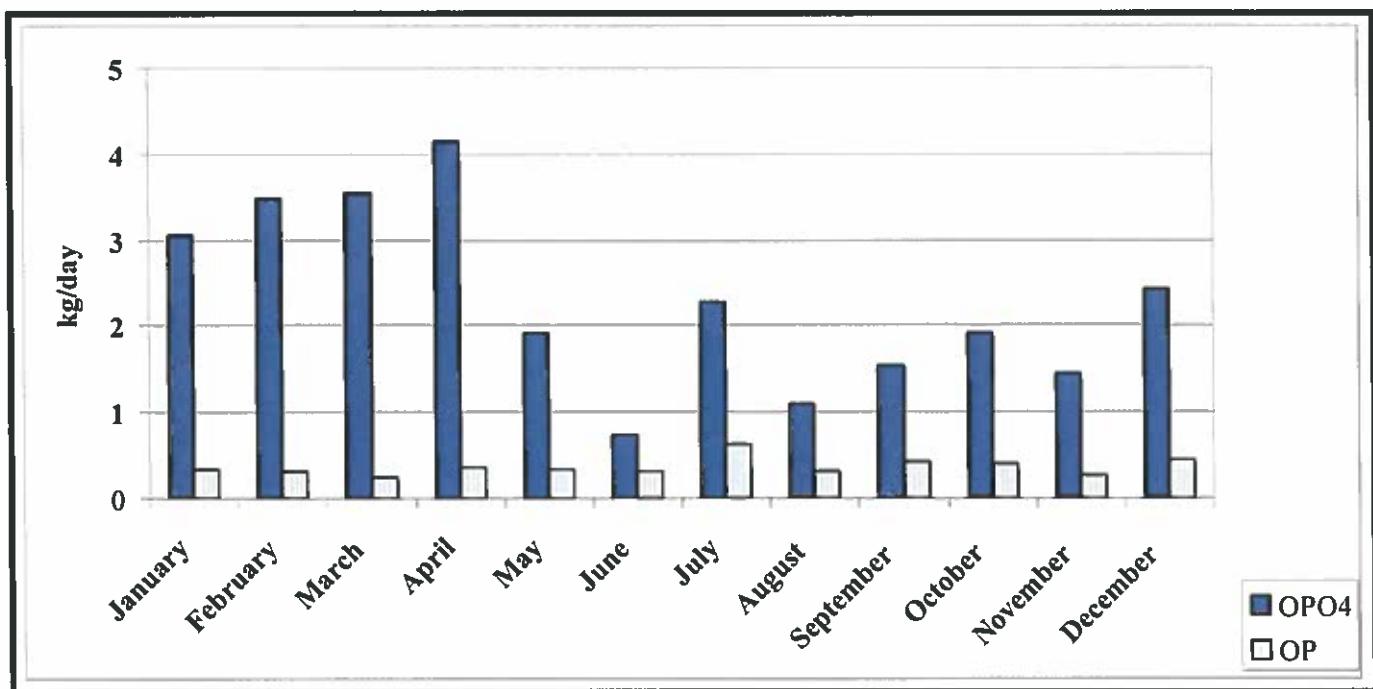


Figure VII-2: Mabank Effluent Total Phosphorous Monthly Average Loads

PROCESS OVERVIEW

The City's WWTP is a conventional wastewater treatment plant utilizing screening, aeration basins, stabilization ponds, and nitrifying towers for treatment. See Figure VII-3 for the plant's process schematic. Table VII-4 lists the existing treatment units and their current sizes.

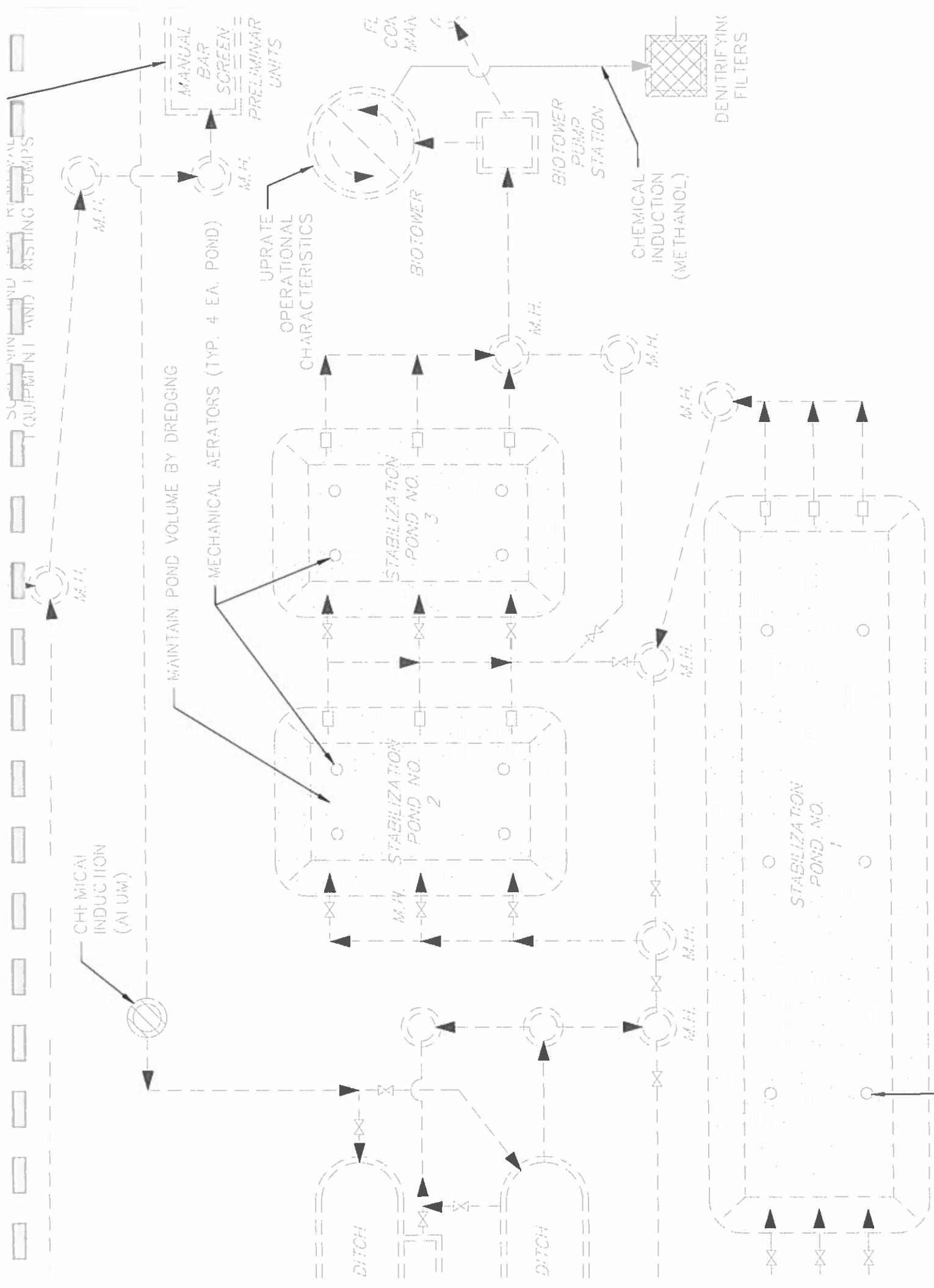
**Table VII-4
City of Mabank WWTP
Existing Treatment Facilities**

Treatment Units	Dimension/Size
Oxidation Ditch Volume	240,000 gallons
Stabilization pond No. 1 Volume	6.63 MG
Stabilization pond No. 2 Volume	1.82 MG
Stabilization pond No. 3 Volume	1.82 MG
Diameter of Biotower	34 feet
Media Depth of Biotower	16 feet

Wastewater flows under gravity into the plant and is routed through a manually cleaned bar screen followed by an automatic grit removal system. The collected solids and grit are disposed of at an off-site landfill facility. An influent lift station pumps the screened and degritted wastewater to an oxidation ditch. The oxidation ditch is operated as pre-aeration basins only. There is no return sludge system. The aerated wastewater is then routed to three stabilization ponds, which have an approximately 30-day retention time. The stabilization ponds are operated as a low-rate facultative ponds without aeration. Sludge is periodically removed from the ponds by dredging and disposed offsite. The ammonia nitrogen is removed from the effluent wastewater through a nitrifying biotower. A Parshall flume measures the effluent before discharging into the creek.

FIGURE VII-3

MECHANICAL AERATORS (TYP. 6)



FUTURE FLOW PROJECTIONS

In the year 2000, the population of the City of Mabank was 2,151. Projected populations and flows through the year 2050 are listed in Table VII-5.

Table VII-5
City of Mabank
Population and Flow Projections

Year	2005	2010	2020	2030	2040	2050
Population ¹	2,400	2,708	3,254	3,814	4,433	5,199
Projected Flow (MGD) ²	0.24	0.27	0.33	0.38	0.44	0.52

¹ Texas Water Development Board 2006 Regional Water Plan

² Average Flow = 100 gal/capita/day (Figure 1: 30 TAC 317.4 (a))

The projected flow will exceed the current plant capacity in the decade between 2030 and 2040.

FACILITY NEEDS EVALUATION

This section describes the facilities identified for the plant to meet each of the three levels of treatment for flows through the year 2050. It should be noted that many of the existing facilities would be beyond their useful lives by the year 2050. The identification of the facility needs does not include the replacement of worn out equipment or treatment units.

Level I Facility Needs for 2050 Wastewater Flows

Level I treatment requirements are identified based on continuing to meet its existing effluent limits. As mentioned above, the plant's current permitted daily average is 0.4 MGD. During the decade between 2030 and 2040, the projected flow will exceed the existing plant capacity, and there will be a need to expand the plant.

To treat year 2050-projected flows, the existing plant needs to be expanded to 0.52 MGD. An additional capacity of 0.08 MGD needs to be added. The headworks structure, consisting of a manually cleaned bar screen, grit removal unit, and a pump station, needs to be expanded to handle higher flows. The existing headworks structure is designed for a peaking factor of 2.5 or a flow of 1 MGD. The headworks structure and influent pump station need to be expanded to handle the peak flow of 1.3 MGD.

Maintaining the current volume of the stabilization ponds and adding surface aerators in each of the ponds should provide the required treatment capacity for the 2050 projected flows under current discharge limits. Four 30-hp surface aerators in each pond are recommended. It should be emphasized that the volumes of the ponds must be maintained through regularly scheduled dredging for solids removal.

The existing biotower is designed as a nitrifying biotower with a low loading rate. The existing biotower has the capacity to treat the year 2050 projected flows. Since the detention time in the stabilization ponds will be reduced at the higher flows, the effluent from the biotower will need to be disinfected once the flow exceeds 1 MGD. A chlorine disinfection system with chlorine gas cylinders

and chlorine feed equipment is recommended. Alternative disinfection technologies may be considered when the expansion is required.

The costs involved to improve the City's plant to treat the year 2050 flow are listed in Table VII-6, as well as additional annual operation costs.

Table VII-6
City of Mabank WWTP
Facility Needs to Meet Level I Effluent Requirements and 2050 Flows

Item Description		Cost
Fine Screens and Grit Removal		\$17,000
Influent Pumping		\$31,000
Aeration Equipment		\$133,000
Disinfection System		\$33,000
Three Sludge Drying Beds		\$60,000
	Sub-Total	\$274,000
Site Work and Yard Piping	20%	\$54,800
	Sub-Total	\$328,800
Electrical and Instrumentation	20%	\$65,760
	Sub-Total	\$394,560
Contingencies	25%	\$98,640
Contractor OHP	15%	\$59,184
	Total Construction Cost	\$552,384
Engineering	10%	\$55,238
Permitting	5%	\$27,619
	Total Project Cost	\$635,242
Power	120 hp and \$0.12 per kW-hr	\$94,000
Chemical		\$0
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5%
	Total O&M Cost per Year	\$98,700

Level II Facility Needs for 2050 Wastewater Flows

The Level II effluent set facility needs are based on reducing the effluent phosphorus concentration to 1 mg/L, total nitrogen to 10 mg/L, CBOD to 5 mg/L, TSS to 5 mg/L, and ammonia nitrogen to 2 mg/L. The historical effluent data show that total nitrogen concentrations average about 12 mg/L and total phosphorus concentrations average about 4 mg/L. To consistently achieve phosphorus concentrations of less than 1 mg/L, the addition of aluminum sulfate (alum) is recommended. Alum solution will be fed prior to the oxidation ditch to provide the required mixing of the alum solution. The stabilization ponds need to be dredged and regularly maintained to utilize the design pond volume and achieve total nitrogen removal down to less than 10 mg/L.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level II effluent requirements are listed in Table VII-7.

**Table VII-7
City of Mabank WWTP
Facility Needs to Meet Level II Effluent Requirements and 2050 Flows**

Item Description		Cost
Chemical Feed System for Alum		\$55,000
	Sub-Total	\$55,000
Site Work and Yard Piping	20%	\$11,000
	Sub-Total	\$66,000
Electrical and Instrumentation	20%	\$13,200
	Sub-Total	\$79,200
Contingencies	25%	\$19,800
Contractor OHP	15%	\$11,880
	Total Construction Cost	\$110,880
Engineering	10%	\$11,088
Permitting	5%	\$5,544
	Total Project Cost	\$127,512
Power	5 hp and \$0.12 per kW-hr	\$4,000
Chemical		\$1,100
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5%
		\$255
	Total O&M Cost per Year	\$5,355

Level III Facility Needs for 2050 Wastewater Flows

The Level III effluent set facility needs are based on reducing the effluent phosphorus concentration to 0.5 mg/L, total nitrogen to 5 mg/L, CBOD to 5 mg/L, TSS to 5 mg/L, and ammonia nitrogen to 2 mg/L. To achieve the Level III concentrations of phosphorus, higher dosages of alum will be needed.

To meet the effluent total nitrogen limits, a denitrifying filter will be required. Additional carbon may be required at the denitrifying filters to reduce total nitrogen levels to less than 5 mg/L. As with the Level I and II facilities, maintaining the pond volume is also crucial to provide the detention time necessary to produce the required effluent quality.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level III effluent requirements are listed in Table VII-8.

Table VII-8
City of Mabank WWTP
Facility Needs to Meet Level III Effluent Requirements and 2050 Flows

Item Description		Cost
Chemical Feed System for Methanol		\$30,000
Denitrification Filters		\$330,500
	Sub-Total	\$360,500
Site Work and Yard Piping	20%	\$72,100
	Sub-Total	\$432,600
Electrical and Instrumentation	20%	\$86,520
	Sub-Total	\$519,120
Contingencies	25%	\$129,780
Contractor OHP	15%	\$77,868
	Total Construction Cost	\$726,768
Engineering	10%	\$72,677
Permitting	5%	\$36,338
	Total Project Cost	\$835,783
Power	20 hp and \$0.12 per kW-hr	\$16,000
Chemical		\$13,800
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5%
		\$1,490
	Total O&M Cost per Year	\$31,290

SECTION VIII

**CITY OF TERRELL
KING'S CREEK WASTEWATER TREATMENT PLANT**

SECTION VIII

CITY OF TERRELL KING'S CREEK WASTEWATER TREATMENT PLANT

INTRODUCTION

The Terrell WWTP discharges into King's Creek and then it flows into Cedar Creek Reservoir in Segment 0818 of the Trinity River Basin. Sources of information for the evaluation included: a site visit, an interview with the plant personnel, review of the existing plans and historical reports, data collected by plant personnel for the District, and the data acquired through Texas Commission on Environmental Quality (TCEQ). This section is organized as follows:

- Description of the existing TPDES permit conditions and projected future limits
- Historical effluent data
- Brief description of the treatment process
- Population growth and flow projections
- Facility needs and costs to meet Level I effluent limits through 2050
- Facility needs and costs to meet Level II effluent limits through 2050
- Facility needs and costs to meet Level III effluent limits through 2050

EXISTING PERMIT CONDITIONS

The City of Terrell operates its wastewater treatment plant under TPDES Permit No. 10747-001 and has a rated capacity of 4.5 million gallons per day (MGD). The permit limits are shown in Table VIII-1; limits shown are existing permit limits, which may have changed throughout the course of historical data collection and report writing.

Table VIII-1
City of Terrell King's Creek WWTP
Current TPDES Permit Conditions
TPDES Permit No. 10747-001

Parameters	Permit Limits
Carbonaceous Biochemical Oxygen Demand (CBOD)	10 mg/L
Total Suspended Solids (TSS)	15 mg/L
Ammonia Nitrogen (NH ₃)	6 mg/L
Flow	4.5 MGD

The existing permit conditions are referred to as Level I. The next level of treatment, referred to as Level II, includes reduction of phosphorus to 1 mg/L and total nitrogen to 10 mg/L. The most stringent level of treatment considered, referred to as Level III, includes reduction of phosphorus to 0.5 mg/L and total nitrogen to 5 mg/L. The assessment of facility needs is performed at all three levels for flows projected for 2050.

HISTORICAL PLANT FLOWS AND EFFLUENT QUALITY

A summary of historical flows and effluent quality for the King's wastewater treatment plant are presented in Table VIII-2; all data in this table were derived from the plant Discharge Monitoring Reports (DMRs). As shown, the flow to the plant is currently about two-thirds of its rated capacity. The plant is consistently meeting its effluent permit conditions.

Table VIII-2
City of Terrell King's Creek WWTP
Historical Plant Flows and Effluent Quality

Year	Average Daily Flow	Effluent		
		CBOD (mg/L)	TSS (mg/L)	NH ₃ -N (mg/L)
	(MGD)	(mg/L)	(mg/L)	(mg/L)
1999	2.4	8.2	5.9	4.4
2000*	2.0	9.2	8.4	6.3
2001	ND	ND	ND	ND
2002	ND	ND	ND	ND
2003*	2.8	9.2	7.7	5.0

ND: No Data
 *Incomplete year's worth of data; average of seven and eleven months, respectively

At the request of the Tarrant Regional Water District, the city of Terrell collected 12 months of effluent nutrient data. Operational and facility changes will be required to meet Level II and III effluent goals. A summary of the data is presented in Table VIII-3; all data may be found in Appendix A.

Table VIII-3
City of Terrell King's Creek WWTP
Special Nutrient Testing for Tarrant Regional Water District

	NH ₃ -N	NO _x	IP	Org P	Org N	TN	TP
No. of Data Points	52	52	52	52	52	52	52
Average (mg/L)	4.50	11.86	4.10	0.24	3.35	19.71	4.03
Minimum (mg/L)	0.62	2.27	1.70	0.00	0.00	9.77	0.72
Maximum (mg/L)	9.10	53.50	6.21	1.95	9.90	62.50	5.63

Figures VIII-1 and VIII-2 illustrate the seasonal variability of total nitrogen and phosphorous loads.

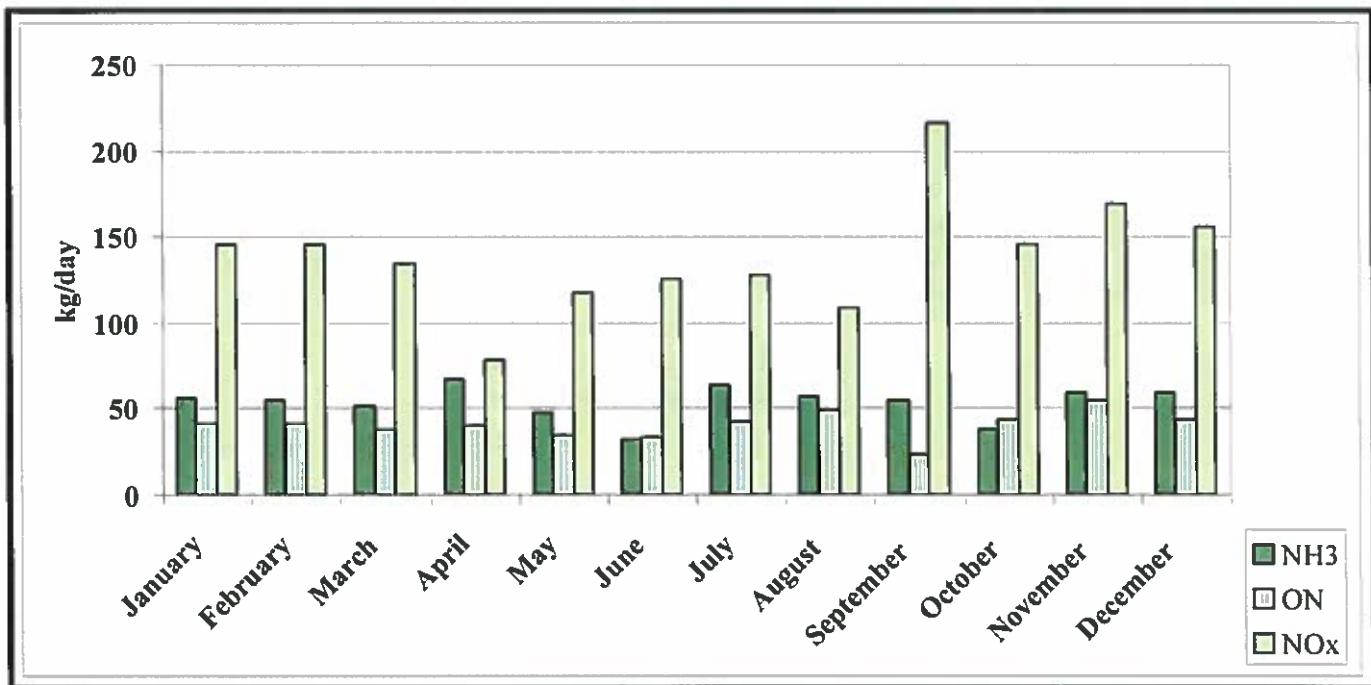


Figure VIII-1: Terrell Effluent Total Nitrogen Monthly Average Loads

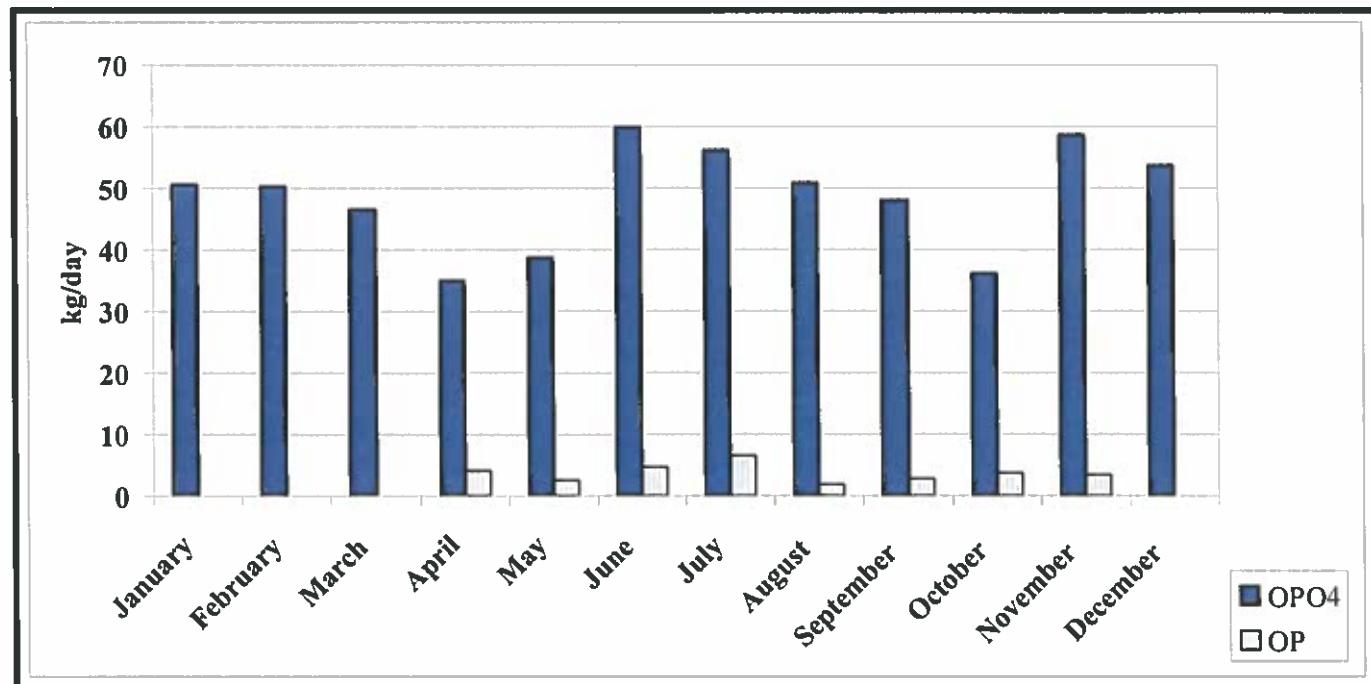


Figure VIII-2: Terrell Effluent Total Phosphorous Monthly Average Loads

PROCESS OVERVIEW

The King's Creek WWTP is a conventional wastewater treatment plant utilizing equalization, screening, grit removal, primary clarification, trickling filters, final clarification, and disinfection for treatment. See Figure VIII-3 for the plant's process schematic. Table VIII-4 lists the existing treatment units and their current sizes.

Table VIII-4
City of Terrell King's Creek WWTP
Existing Treatment Facilities

Treatment Units	Dimension/Size
Equalization Basin Volume	0.66 million gallons
Primary Clarifier Volume	85-ft diameter, 7-ft SWD 297,000 gallons
First Trickling Filter Volume	140-ft diameter, 7-ft SWD 806,000 gallons
Intermediate Clarifier Volume	75-ft diameter, 7-ft SWD 231,000 gallons
Second Trickling Filters 1 and 2 Volume	100-ft diameter, 6-ft SWD 352,000 gallons
Final Clarifiers 1 and 2 Volume	85-ft diameter, 10-ft SWD 424,000 gallons (each)
Chlorine Contact Basin Volume	147,500 gallons
Anaerobic Digesters 1 and 2 Volume	0.27 MG
Sludge Holding Tank Volume	0.13 MG

Wastewater flows under gravity to an equalization basin and then to the headworks. Headworks structure utilizes a mechanically cleaned bar screen with $\frac{1}{4}$ -inch opening to remove larger solids and two aerated grit removal chambers. The screenings and grit are disposed at an off-site landfill facility.

Wastewater is clarified under gravitational force in the primary clarification basin to separate a portion of the suspended solids and organic matter from the wastewater. The King's Creek WWTP uses a two-stage trickling filter system. In this system, wastewater trickles through a bed of plastic packing materials, to which the microorganisms are attached, and then passes through a clarification unit. The clarified wastewater passes through a second-stage trickling filter and to another clarifier for settling. The first-stage trickling filter is primarily utilized to reduce the carbonaceous biochemical oxygen demand in the wastewater and the second-stage trickling filter reduces the concentrations of ammonia nitrogen present in the wastewater. A portable belt press serves to dewater the sludge.

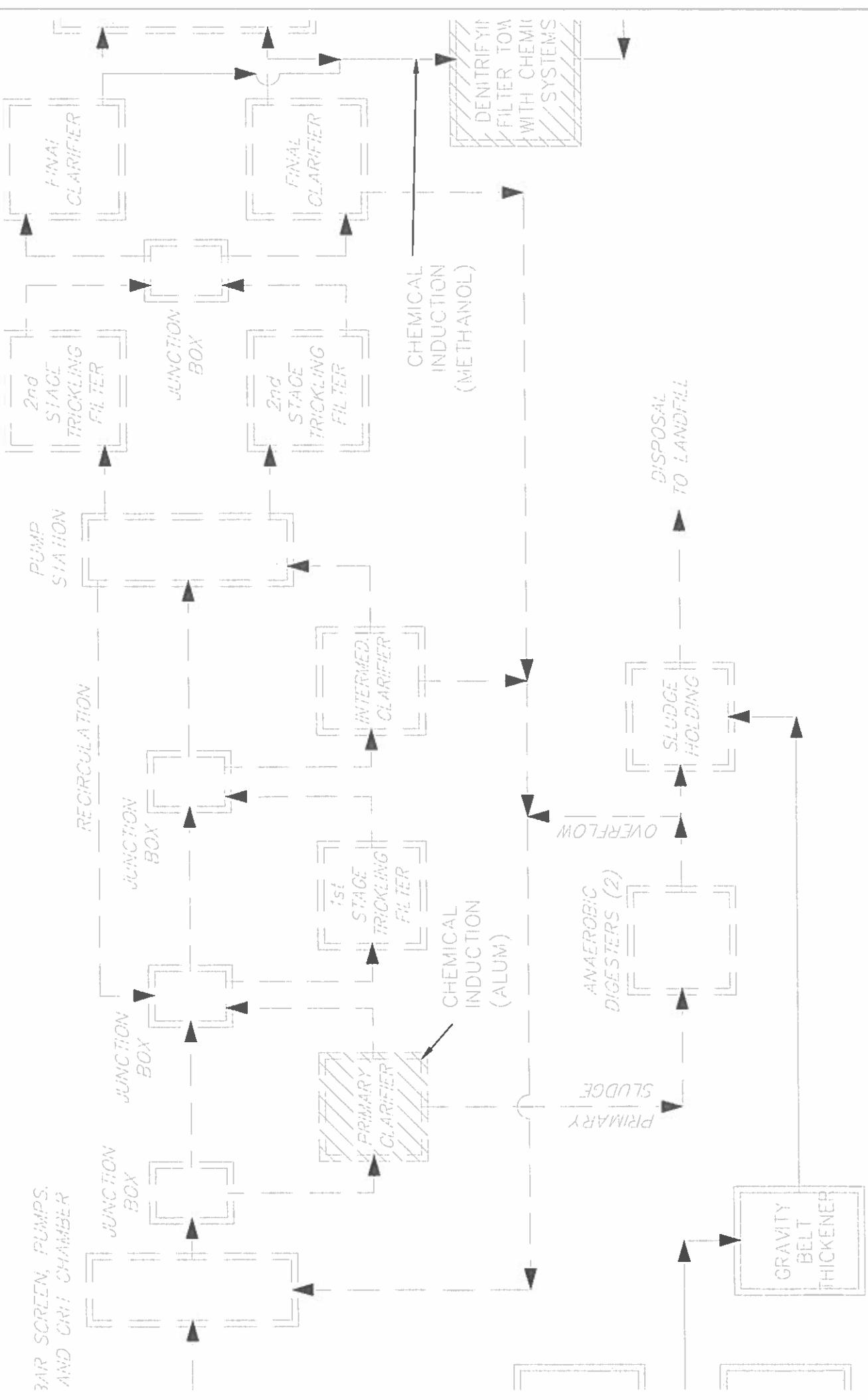


FIGURE VIII-3

WATER TREATMENT PLANT

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FUTURE FLOW PROJECTIONS

In the year 2000, the population of the City of Terrell was 13,606. Projected populations and flows through the year 2050 are listed in Table VIII-5.

Table VIII-5
City of Terrell
Population and Flow Projections

Year	2005	2010	2020	2030	2040	2050
Population ¹	14,379	15,196	18,642	21,664	23,650	25,599
Projected Flow (MGD) ²	3.23	3.42	4.19	4.87	5.32	5.76

¹ Texas Water Development Board 2006 Regional Water Plan

² Average Flow = 135 gal/Capita/day (Observed by the plant) and 40% Industrial Flow

The King's Creek WWTP has historically received more flow than the TCEQ design criteria of 100 gal/capita/day. The 135 gal/capita/day observed by the plant was used to project flows. The projected flows will exceed the current plant capacity of 4.5 MGD around the year 2025.

FACILITY NEEDS EVALUATION

This section describes the facilities identified for the plant to meet each of the three levels of treatment for flows through the year 2050. It should be noted that many of the existing facilities will be beyond their useful lives by the year 2050. The identification of the facility needs does not include the replacement of worn out equipment or treatment units.

Level I Facility Needs for 2050 Wastewater Flows

Level I treatment requirements are identified based on continuing to meet its existing effluent limits. As mentioned above, the plant's current permitted daily average flow is 4.5 MGD. To treat higher flows through the year 2050, the existing plant needs to be expanded to add an additional 1.26 MGD of capacity between the decade of 2020 and 2030.

To achieve the expanded capacity, an additional treatment train is required. Fine screens and grit removal are needed for preliminary treatment. Two new sequencing batch reactors (SBRs) and single media automatic backwash filters (ABW) could supply the required capacity to treat the higher flows. The existing chlorine contact basin would be expanded. To handle the higher solids volume, a gravity belt thickener is recommended.

The capital costs, as well as the annual operational costs, involved to improve the City's plant to treat the year 2050 flow are listed in Table VIII-6.

Table VIII-6
City of Terrell King's Creek WWTP
Facility Needs to Meet Level I Effluent Requirements and 2050 Flows

Item Description		Cost
Fine Screens and Grit Removal		\$218,000
Sequencing Batch Reactors		\$1,759,000
Automatic Backwash Filters		\$724,000
Expansion of Chlorine Contact Basin		\$277,000
Gravity Belt Thickener		\$221,000
	Sub-Total	\$3,199,000
Site Work and Yard Piping	20%	\$639,800
	Sub-Total	\$3,838,800
Electrical and Instrumentation	20%	\$767,760
	Sub-Total	\$4,606,560
Contingencies	25%	\$1,151,640
Contractor OHP	15%	\$690,984
	Total Construction Cost	\$6,449,184
Engineering	10%	\$644,918
Permitting	5%	\$322,459
	Total Project Cost	\$7,416,562
Power	120 hp and \$0.12 per kW-hr	\$94,000
Chemical		\$2,300
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5%
	Total O&M Cost per Year	\$4,815
		\$101,115

Level II Facility Needs for 2050 Wastewater Flows

The Level II effluent set facility needs are based on reducing the effluent phosphorus concentration to 1 mg/L, total nitrogen to 10 mg/L, CBOD to 10 mg/L, TSS to 15 mg/L, and ammonia nitrogen to 6 mg/L. As detailed in Table VIII-3, the average effluent total nitrogen concentration was about 20 mg/L and the average effluent total phosphorus concentration was about 4 mg/L.

To consistently achieve total phosphorus concentrations of 1 mg/L, add aluminum sulfate (alum) to precipitate with the phosphorous in the wastewater. To consistently achieve a total nitrogen effluent concentration of less than 10 mg/L, add a denitrifying filter to the existing treatment train and operate the new SBRs for denitrification.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level II effluent requirements are listed in Table VIII-7.

Table VIII-7
City of Terrell King's Creek WWTP
Facility Needs to Meet Level II Effluent Requirements and 2050 Flows

Item Description		Cost
Chemical Feed System for Alum		\$55,000
Denitrification Filters		\$1,322,000
	Sub-Total	\$1,377,000
Site Work and Yard Piping	20%	\$275,400
	Sub-Total	\$1,652,400
Electrical and Instrumentation	20%	\$330,480
	Sub-Total	\$1,982,880
Contingencies	25%	\$495,720
Contractor OHP	15%	\$297,432
	Total Construction Cost	\$2,776,032
Engineering	10%	\$277,603
Permitting	5%	\$138,802
	Total Project Cost	\$3,192,437
Power	5 hp and \$0.12 per kW-hr	\$4,000
Chemical		\$11,100
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5%
		\$755
	Total O&M Cost per Year	\$15,855

Level III Facility Needs for 2050 Wastewater Flows

The Level III effluent set facility needs are based on reducing the effluent phosphorus concentration to 0.5 mg/L and total nitrogen to 5 mg/L. To achieve the Level III concentrations of phosphorus, higher dosages of alum will be needed. The denitrifying filter required for Level II can reduce the total nitrogen to the Level III limits. Additional carbon may be required at the denitrifying filters to reduce total nitrogen levels to less than 5 mg/L.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level III effluent requirements are listed in Table VIII-8.

Table VIII-8
City of Terrell King's Creek WWTP
Facility Needs to Meet Level III Effluent Requirements and 2050 Flows

Item Description		Cost
Chemical Feed System for Methanol		\$63,000
	Sub-Total	\$63,000
Site Work and Yard Piping	20%	\$12,600
	Sub-Total	\$75,600
Electrical and Instrumentation	20%	\$15,120
	Sub-Total	\$90,720
Contingencies	25%	\$22,680
Contractor OHP	15%	\$13,608
	Total Construction Cost	\$127,008
Engineering	10%	\$12,701
Permitting	5%	\$6,350
	Total Project Cost	\$146,059
Power		
Chemical		\$170,000
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5%
		\$8,500
	Total O&M Cost per Year	\$178,500

SECTION IX

CITY OF WILLS POINT

WASTEWATER TREATMENT PLANT

SECTION IX

CITY OF WILLS POINT WASTEWATER TREATMENT PLANT

INTRODUCTION

The Wills Point WWTP discharges into Little Allen Creek; thence Allen Creek; thence Cedar Creek; thence Cedar Creek Reservoir in Segment 0818 of the Trinity River Basin. Sources of information for the evaluation included: a site visit, an interview with the plant personnel, review of the existing plans and historic reports, data collected by plant personnel for the District, and the data acquired through Texas Commission on Environmental Quality (TCEQ). This section is organized as follows:

- Description of the existing TPDES permit conditions
- Historical effluent data
- Brief description of the treatment process
- Population growth and flow projections
- Facility needs and costs to meet Level I effluent limits through 2050
- Facility needs and costs to meet Level II effluent limits through 2050
- Facility needs and costs to meet Level III effluent limits through 2050

EXISTING PERMIT CONDITIONS

The City of Wills Point operates its wastewater treatment plant under TPDES Permit No. 10623-001. The current permitted capacity is 0.51 million gallons per day (MGD). The City has established plans to expand the plant to treat and average daily flow of 0.80 MGD. The permit limits are shown in Table IX-1; limits shown are existing permit limits, which may have changed throughout the course of historical data collection and report writing.

Table IX-I
City of Wills Point WWTP
Current TPDES Permit Conditions
TPDES Permit No. 10623-001

Parameters	Permit Limits
	<u>Existing/Interim</u>
Biochemical Oxygen Demand (BOD ₅)	30 mg/L
Total Suspended Solids (TSS)	90 mg/L
Flow	0.51 MGD
	<u>Future</u>
Carbonaceous Biochemical Oxygen Demand (CBOD)	10 mg/L
Total Suspended Solids (TSS)	15 mg/L
Ammonia Nitrogen	3 mg/L
Flow	0.80 MGD

The permit conditions after the planned expansion are referred to as Level I. The next level of treatment, referred to as Level II, includes reduction of phosphorus to 1 mg/L and total nitrogen to 10 mg/L. The most stringent level of treatment considered, referred to as Level III, includes reduction of

phosphorus to 0.5 mg/L and total nitrogen to 5 mg/L. The assessment of facility needs is performed at all three levels for flows projected for 2050.

HISTORICAL PLANT FLOWS AND EFFLUENT QUALITY

A summary of the historical flows and effluent quality for the City of Wills Point wastewater treatment plant are presented in Table IX-2; all data in this table were derived from the plant Discharge Monitoring Reports (DMRs). As shown, the flow to the plant is currently about 80% of its present rated capacity, which explains the need to expand to 0.8 MGD. As of the writing of this report, the plant was still operating under the interim permit limits.

Table IX-2
City of Wills Point WWTP
Historical Plant Flows and Effluent Quality

Year	Average Daily Flow	Effluent	
		BOD ₅	TSS
	(MGD)	(mg/L)	(mg/L)
1999	0.45	27.5	73.8
2000	0.44	32.5	89.0
2001*	0.54	22.5	68.0
2002	0.42	28.6	79.0
2003*	0.38	29.4	79.0

*Incomplete year's worth of data; average of ten and eleven months, respectively

The plant is usually meeting interim effluent permit limits for BOD₅. At the request of the Tarrant Regional Water District, the city of Wills Point collected 12 months of effluent nutrient data. Operational and facility changes will be required to meet Level II and III effluent goals. A summary of the data is presented in Table IX-3; all data may be found in Appendix A.

Table IX-3
City of Wills Point WWTP
Special Nutrient Testing for Tarrant Regional Water District

	NH ₃ -N	NO _x	IP	Org P	Org N	TN	TP
No. of Data Points	49	49	49	49	49	49	49
Average (mg/L)	3.15	1.55	1.72	1.17	7.33	12.03	2.64
Minimum (mg/L)	0.18	0.08	0.09	0.00	0.56	0.87	0.72
Maximum (mg/L)	9.98	6.57	3.88	2.57	36.00	36.48	4.45

Figures IX-1 and IX-2 illustrate the seasonal variability of total nitrogen and phosphorous loads.

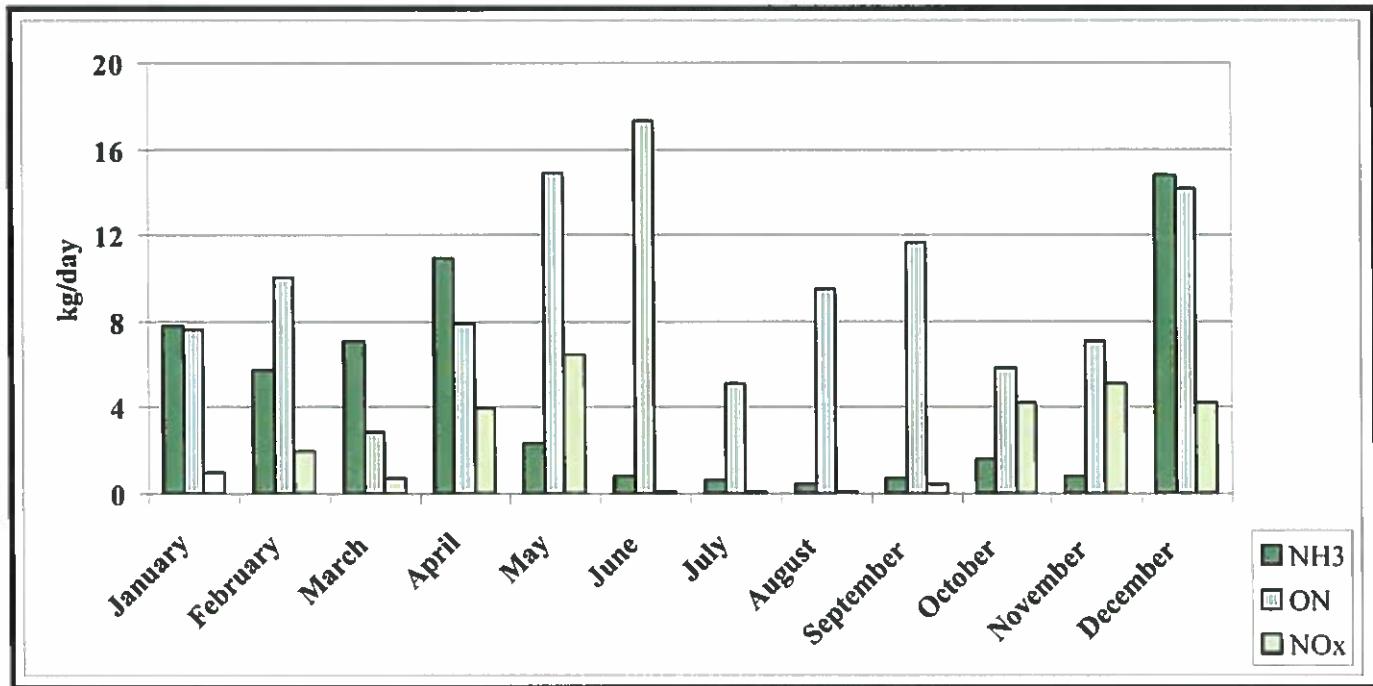


Figure IX-1: Wills Point Effluent Total Nitrogen Monthly Average Loads

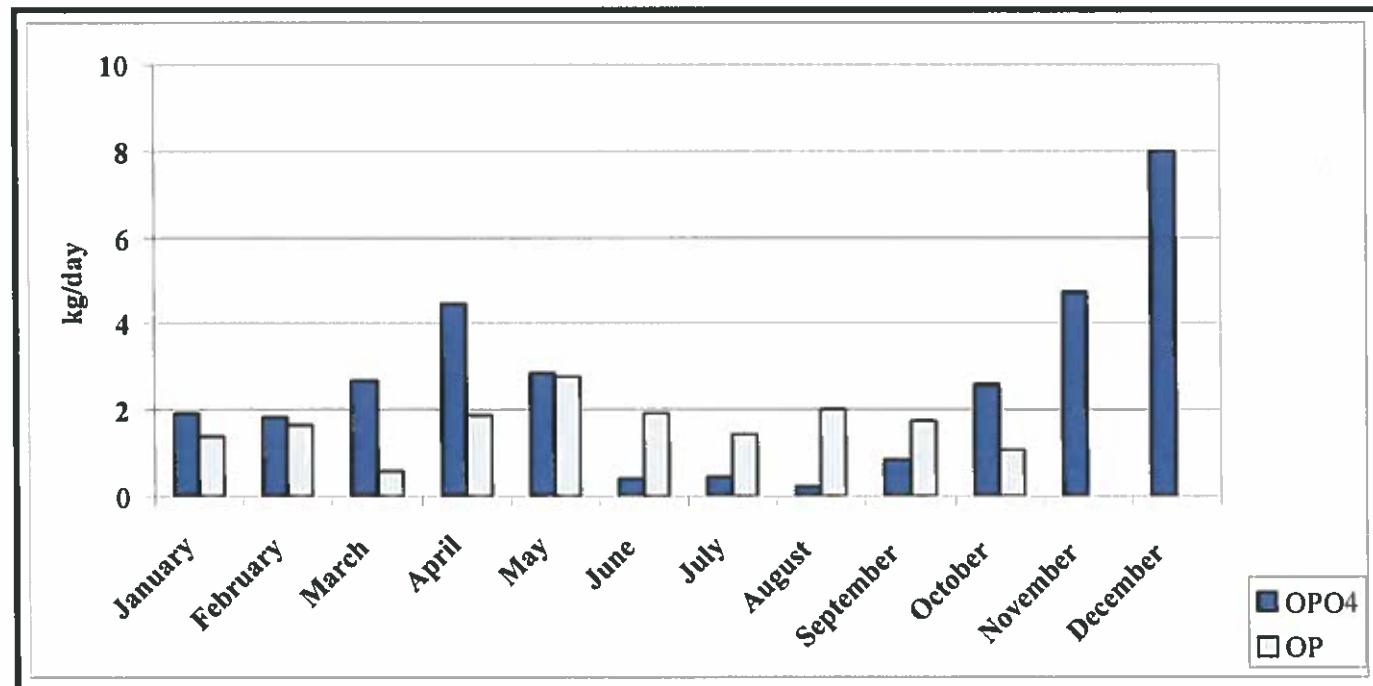


Figure IX-2: Wills Point Effluent Total Phosphorous Monthly Average Loads

PROCESS OVERVIEW

See Figure IX-3 for the plant's process schematic. Table IX-4A and Table IX-4B list the existing treatment units and their current and planned sizes, respectively.

**Table IX-4A
City of Wills Point WWTP
Existing Treatment Facilities**

Treatment Units	Dimension/Size
Facultative Pond No. 1 Volume	5.43 MG
Facultative Pond No. 2 Volume	4.90 MG
Stabilization Pond No. 3 Volume	9.94 MG
Stabilization Pond No. 4 Volume	14.50 MG
Stabilization Pond No. 5 Volume	7.38 MG

**Table IX-4B
City of Wills Point WWTP
Planned Treatment Facilities**

Treatment Units	Dimension/Size
Stabilization Pond No. 6 Volume	3.38 MG
Stabilization Pond No. 7 Volume	3.42 MG

The City of Wills Point sewer collection system brings wastewater to the headworks of the plant. The headworks structure consists of influent junction box, a manually cleaned bar screen, a Parshall flume to measure the influent, and two lift stations. From the headworks, the wastewater is pumped to facultative ponds and then the wastewater flows by gravity through a series of stabilization ponds. The facultative lagoons have two layers: the top/surface layer is aerobic and bottom layer is anaerobic. Oxygen is supplied to the surface layer by the natural diffusion and photosynthetic algae. The sludge settled in the facultative and stabilization ponds is periodically dredged, dried and disposed in a sanitary landfill. A Parshall flume meters the effluent prior to discharge.

FIGURE IX-3



FUTURE FLOW PROJECTIONS

In the year 2000, the population of the City of Wills Point was 3,496. Projected populations and flows through the year 2050 are listed in Table IX-5.

**Table IX-5
City of Wills Point
Population and Flow Projections**

Year	2005	2010	2020	2030	2040	2050
Population ¹	3,700	3,860	4,243	4,566	4,809	5,117
Projected Flow (MGD) ²	0.37	0.39	0.42	0.46	0.48	0.51

¹ Texas Water Development Board 2006 Regional Water Plan

² Average Flow = 100 gal/capita/day (Figure 1, 30 TAC 317.4 (a))

The current capacity of the plant is able to treat the projected flows through the year 2050.

FACILITY NEEDS EVALUATION

This section describes the facilities that are required for the plant to meet each of the three levels of treatment for flows through the year 2050. It should be noted that many of the existing facilities will be beyond their useful lives by the year 2050. The identification of the facility needs does not include the replacement of worn out equipment or treatment units.

Level I Facility Needs for 2050 Wastewater Flows

Level I treatment requirements are identified based on continuing to meet its existing effluent limits. As mentioned above, the permitted daily average flow through the City's plant is 0.80 MGD, which is more than the projected flow through the year 2050. No additional modifications and/or expansions are required.

Level II Facility Needs for 2050 Wastewater Flows

The Level II effluent set facility needs are based on reducing the effluent phosphorus concentration to 1 mg/L and total nitrogen to 10 mg/L. As detailed in Table IX-3, the historical average total nitrogen effluent concentration was about 12 mg/L, and the historical average total phosphorus concentration was about 3 mg/L. To consistently achieve a total nitrogen effluent concentration of less than 10 mg/L, an earthen pond separator or dyke should be constructed in the first facultative pond to create a new 1 million gallon pond at the inlet to the facultative pond system. The stabilization ponds need to be dredged often to maintain and utilize the design pond volume.

To consistently achieve total phosphorus effluent concentrations of less than 1 mg/L, the addition of aluminum sulfate (alum) is recommended. Alum solution will be fed at the influent pump station that would provide the required mixing of the alum solution.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level II effluent requirements are listed in Table IX-6.

Table IX-6
City of Wills Point WWTP
Facility Needs to Meet Level II Effluent Requirements and 2050 Flows

Item Description		Cost
Dredging Ponds		\$50,000
Chemical Feed System for Alum		\$22,000
Earthen Pond Separator		\$28,000
	Sub-Total	\$100,000
Site Work and Yard Piping	20%	\$20,000
	Sub-Total	\$120,000
Electrical and Instrumentation	20%	\$24,000
	Sub-Total	\$144,000
Contingencies	25%	\$36,000
Contractor OHP	15%	\$21,600
	Total Construction Cost	\$201,600
Engineering	10%	\$20,160
Permitting	5%	\$10,080
	Total Project Cost	\$231,840
Power	5 hp and \$0.12 per kW-hr	\$4,000
Chemical		\$700
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5%
		\$235
	Total O&M Cost per Year	\$4,935

Level III Facility Needs for 2050 Wastewater Flows

The Level III effluent set facility needs are based on reducing the effluent phosphorus concentration to 0.5 mg/L and total nitrogen to 5 mg/L. To achieve the Level III concentrations of phosphorus, higher dosages of alum will be needed. To meet the effluent total nitrogen limits, a denitrifying filter is required. Additional carbon may be required at the denitrifying filters to reduce total nitrogen levels to less than 5 mg/L. As with the Level I and II facilities, maintaining the pond volume is also crucial to provide the biological treatment necessary to produce the required effluent quality.

The planning level estimates of probable costs involved to improve the City's plant to treat the year 2050 flow to Level III effluent requirements are listed in Table IX-7.

Table IX-7
City of Wills Point WWTP
Facility Needs to Meet Level III Effluent Requirements and 2050 Flows

Item Description		Cost
Chemical Feed System for Methanol		\$30,000
Denitrification Filter		\$330,500
	Sub-Total	\$360,500
Site Work and Yard Piping	20%	\$72,100
	Sub-Total	\$432,600
Electrical and Instrumentation	20%	\$86,520
	Sub-Total	\$519,120
Contingencies	25%	\$129,780
Contractor OHP	15%	\$77,868
	Total Construction Cost	\$726,768
Engineering	10%	\$72,677
Permitting	5%	\$36,338
	Total Project Cost	\$835,783
Power	20 hp and \$0.12 per kW-hr	\$16,000
Chemical		\$13,800
Labor	No Added Personnel	\$0
Maintenance	Assume 5% of additional operation cost	5% \$1,490
	Total O&M Cost per Year	\$31,290

APPENDIX A

